# "Restoration and Regradation of Giruthumal river in Madurai, Sivagangai, Virudhunagar and Ramanathapuram district."

#### **INTRODUCTORY:**

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The Gundar river takes rise from the eastern slope of Varusanadu Hills at an altitude of 1273m near Kottaimalai of Saptur reserve forest on the eastern slopes of Western Ghats in Madurai District and runs southeast for a distance of 150 km and finally empties into Gulf of Mannar at about 6 km of south east of Sayalkudi of Ramanathapuram District. The Gundar river basin is located between latitude 9° 05′ N to 10° 03′ N and longitude 77° 35′ E to 78° 35′ E having an area of 569023 Sq.Km and is surrounded by Vaigai Basin on the South, Vaigai Basin on the West and North and Gulf of Mannar / Bay of Bengal on the east.

The Gundar Basin has been divided into 9 sub basins and Girudhumal is one of the sub basin. As per mythology, the historical town of Madurai was located on the banks of the ever flowing Gridhumal river. The river Gridhumal originates from the field water drainage of Thuvariman and Madakulam Tanks near Madurai city in Thiruppurankunram Block and of Madurai district. It runs to a length of about 86 KMs in Madurai South, Thiruppuvanam, Thiruchuli & Kamuthi Taluks. This river receives this drainage from the city through Avaniapuram and Chinthamani Supply channel running in the middle of the city. The surplus of Konthagai tanks also reaches Gridhumal river.

The Girudhumal Sub basin is located between latitude 9°25′00″ N to 9°50′00″ N and longitude 78°05′00″ E to 78°25′00″ E and is surrounded by Vaigai river on the North and Kanal Odai Sub basin on South. Girudhumal Sub basin area is 566.851 Sq.Km with a plain area. The taluks covered in the sub basin are Madurai South, Thiruppuvanam, Kariapatti, Thiruchuli, Paramakudi, Kamuthi taluks of Madurai, Sivagangai, Virudhunagar and Ramanathapuram District respectively. It receives an annual average rainfall of 739mm, with its major share during North-East Monsoon. The tributary Gridhumal runs through Narikudi, Veeracholan and crosses the Raghunatha Cauvery at Pakkuvetti village in Kamuthi taluk and joins the river Malattar. There are seven anicuts constructed across this river, which feed 73 tanks through several chain of tanks. The river Malattar feeds 54 tanks in Lower Gundar Basin.

## **Details of Ayacut**

There is no direct ayacut from anicuts and reservoir in Gridhumal Sub basin. The system and Non system tanks in Gridhumal sub basin are given below.

<b>S1</b> .		Sy	stem	Non-system			
No.	District	Nos	Ayacut in Ha.	Nos	Ayacut in Ha.		
1.	Madurai	9	982.27	8	982.42		
2.	Sivagangai	22	6161.99	24	3146.23		
3.	Virudhunaga r			41	5042.14		
4	Ramnad			12	1000.46		
		31	7144.26	85	10171.25		

# ANICUTS IN GIRUDHAMAL

The following anicuts are located in the sub basin

- 1 Ambalathadi anicut
- 2 Odathur anicut
- 3 Kattanur anicut
- 4 Athikulam anicut
- 5 Nallukuruchi anicut
- 6 Abiramam anicut
- 7 Ambalathadi new anicut (Under progress in IAMWARM phase 3)

# The total ayacut area under the sub-basin: 17315.51 Ha

Though the total registered ayacut under PWD control is 17315.51 Ha, average cultivation is only in **11809.35 Ha** leaving a gap of **5506.16Ha** which is approximately 32 % of designed irrigation extent. The gap area is to be bridged by the execution of

IAMWARM Project Phase III, under which rehabilitation works for 113 tanks are under progress.

## Water Potential

Total	134.01 Mcum	4733.23 Mcft
Ground water potential	87.25 Mcum	3081.67 Mcft
Surface water potential	46.76 Mcum	1651.56 Mcft

In Gridhumal River Irrigation is carried out through supply channels with open off-takes at various places and also from seven anicuts. The Gridhumal river irrigates an extent of 17315.51 Hec.

The river below Gridhumal namely Malattar irrigates an extent of 8890 acres through 54 tanks in Lower Gundar sub basin.

# **ISSUES AILING IN THE BASIN**

# Non availability of water

The water resource of this basin is reeling under severe drought due to the failure of successive monsoons and is further deprived of its water resources to greater extent by the abundant growth of Juliflora.

"Pheratophybes" are plants that send their roots down to the water table or the capillary fringe just above the water table which provides a ready supply of water. They are water loving plants that grow mainly along stream courses. There they are able to get lasting supply of water. One of the dominant species responsible for most of the heavy use of water is Prosopis Juliflora which occurs in valley bottom, tank beds, vacant lands and along streams.

Prospis Juliflora plants are multi-stemmed shrubby bushes growing from 3m to 15m tall. Juliflora has been know to sent its roots 10,20 or even 30 m to water. Their roots lift water much higher than it can be lifted by capillary action of the soil. They are forming dense, impenetrable thickets combined with large thorns. They also reduces the productivity of pastoral country by taking over grass lands and using water resources. These plants have no fodder value.

#### **Consumption of jungle trees by animals**

Other nuisances include damage to animal hooves from thorns and poisoning of livestock which consume excessier amount of seed pods. It is reported tha consumption of water by adense growth of (Mesquite) Prospis juliflor is about to 2 to 5 acre feet derived from ground water, surface water and precipitation. Most of the tank beds in the sub basin are covered with Juliflora growth.

# PRESENT STATUS OF THE RIVER:

The entire length of about 20 KM of its passage in and around the Madurai City and Sub-urban area were functioning as a domestic drainage carrier.

Now a quantity of 1000 cusecs flood water of Vaigai river is being diverted through the Flood Carrrier Head Sluice above the Virahanur regulator to this Gridhumal.

The Girdhumal feeds 73 Nos. of Tanks which has a total capacity of 1583 Mcft and also it feeds 54 tanks of Lower Gundar Basin with an ayacut of 8890 Acres.

The entire length of the river below the urban area, from 20<sup>th</sup> KM of the river up to 86.56 KM, the confluence point into Regunatha Cauvery is fully covered with thick Juliflora Jungle growth and the bed level of the river varies drastically due to sand mining and shoal formation. The river to its entire length except at anicut sites could not be identified due to jungle growth and without banks. Due to the above mentioned facts the river at present lost its original course and the tanks fed by the river could not receive water. All the tanks under Gridhumal are rehabilitated under IAMWARM Project phase III. To effectively utilize the flood water it is quite essential to improve and modernize the Gridhumal river.

The link canal with head sluice was loacated at right bank of viagai river and upstream of Virahanur regulator for diverting vaigai flood water to Giruthumal river. This head sluice and link canal was designed to divert 1000 cusec of flood water. At present 678 cusecs of flood water only realized due to insufficient driving head.

#### DETAILS OF PROPOSALS FOR IMPROVING THE IRRIGATION:

To restore and to re-grade the river for improving the inflow to tanks the following items of works are proposed.

#### 1. Improving the effiency of the link channel

There is a link canal of length 9600m existing at LS560m above the Viraganur regulator across vagai river to divert flood water from Vagai river to Girudamal river. It was originally designed to carry 1000 cusecs. Due to insufficient driving head at present only 678cusecs were realized. Hence it is proposed to increase the no. of existing vents of link canal head sluice from 5 to 7 there by achieving the designed discharge of 1000cusecs. It is also proposed to increase the canal efficiency by lining with in-situ cement concrete of grade M-15 for a length of 5490m from the commencing point and stone pitching for a length of 110 m from the end of lined portion and side slopes are also proposed to increased from 1V :1 H to 1V :1.5 H . Apart from this desilting the canal for a length of 4000m is also proposed. When the driving head is increased it can result in flooding of fields adjacent to Vagai river. Hence it is necessary to raise the existing bund level to M F L of Virahanur regulator for a length of 2km on left side and 1 Km on Right side and regradation of vaigai river bed for a distance of about 1.10 km upstream of virahanur regulator and this is also included. The necessary designs have been obtained from Superintending Engineer, Designs circle office, Chennai.

#### 2.0 Bed re-gradation and bund formation.

Due to the abundant growth of Juiliflora (Prosopis) over the entire length and breadth of river and due to the degradation of the bed, the water could not reach the tail end. To effecitively utilize the water it is proposed to regrade the river bed by excavating the river bed using earth moving machineries and forming the hearting zone of earthern bund on eitherside of the river by the excavated earth. Excessive surplus excavated earth was proposed to be dumped in the spoil bank. Casing layer of suitable earth for bund formation, from borrow areas with a lead of 5 Km is proposed to be conveyed and compacted using power roller.

#### 3.0 Construction of Anicuts at open off take points across Gridhumal river

It is proposed to construct 4 nos. of anicuts at the following open off- take points across Gridhumal river.

- 1. Sangankulam open off take
- 2. Varisaiyur open off take
- 3. Keelaparithiyur open off take
- 4. Kallikulam open off take

The irrigation is being practiced through open off take by the farmers. Since the commandability is not achieved, they are in defunct condition. The head causing flow through open off take are meagre. Now it is proposed to construct anicuts to restore the bed level of the river as well as to feed water through the head sluices.

#### 4. Construction of protection walls:

It is proposed to construct protection walls along the bunds at the vulnerable points where the width of the river is narrow . The protection walls are proposed in cement concrete M15 grade using graded metal necessarytransition arrangements and bank connections were also provided with core walls in M15 Grade concrete. The details of protection walls and bank connection walls were designed by CE,DRC&S and communicated Vide Lr No: 299CE(D)/SE(D)/AEE X/F563/2012/ Dt: 27.12.12.

#### 5. Fixing boundary stones including sureveying and leveling operations:

Boundary stones are proposed to be fixed along the boundaries of the river. Surveying and leveling operations are proposed to fix the boundary of the river.

# 6. Creation of alternate water source to Girudhumal river

The three vaigai fed tanks namely Kondagai, Piramanur and Palayanur are situated between Girudamal and Vagai river and downstream of link canal from Vagai. The surplus of above tanks goes to Girudhumal river. As alternate sources to Girudhumal the following are details

• It is proposed to draw surplus water for a discharge of 430 cusecs and 415 cusecs respectively by repairing scour vent in Kondagai tank and Providing new scour vent in Palayanur tank.

All the necessary designs were obtained from Superintending Engineer, Designs circle office, Chennai.

# 7.0 INITIAL INVESTIGATIONS CARRIED OUT

Longitudinal and cross sectional plan at 100m intervals for the entire reach of 66.56Km are taken and based on that cross section of the river for various reaches were worked out by the designs wing and accordingly detailed estimate was prepared. Detailed drawings are enclosed.

Designs and drawing for increasing the no. of vents from 5 to 7 for the Link canal Head sluice was obtained from the designs wing accordingly the estimate was prepared.

It is proposed to improve the link canal by lining the bed and sides of canal for which the design was prepared by the designs wing based on that detailed estimate was prepared.

It is proposed to construct 5No. of anicuts at the following off-take points 1.Sankankulam 2. Varisayur 3.Keelaparithiyur 4.T.Punavasal 5.Kallikulam for which the detailed estimates were prepared based on the designs and drawings of the design wing.

The location for providing protection walls were identified properly and the cost estimate was arrived based on the designed drawings of the designs wing.

It is clearly stated that the proposal does not involve any encroachment and no land acquisition is needed.

The following components are proposed in this Proposal

- 1. Lining of Girudhumal Link Canal by cement concrete lining.
- 2. Construction of Scour vents to the Palayanur Weir.
- 3. Construction of Additional vents to the head sluice of Girudhumal link canal.
- 4. Repair works to Scour sluice of Konthagai tank.
- 5. Repairs to Silaman Head Sluice in Girudhumal link canal.
- 6. Repairs to Melavellore Dividing dam.
- 7. Improvement to existing Flood Banks of Vaigai river.
- 8. Bed regradation and bund formation in Girudhumal river ls 0 m to 66.56 m.
- 9. Repairs to exsisting anicuts
- 10. Construction of anicuts at open off-take points across Girudhumal river
- 11. Construction of protection walls at vulnerable locations.

#### **IMPROVEMENTS EXPECTED**

At present the are 113 tanks situated along the basin. About 20 system tanks receive water only up to Kattanur anicut and because of the highly uneven river reach encroached with thick jungles, the water could not reach the remaining tanks. With the improvement proposed it is possible to cover the balance tanks numbering about 93 numbers.

Due to the above mentioned facts river lost its original course and the tanks fed by river could not receive water. All the tanks under this basin are being rehabilitated under IAMWARM phaseIII project. As the river course condition is very bad, the tanks under renovation will not get water. Hence for proper inflow in the above tanks under rehabilitation, the present project is planned and once completed the tanks will become effective.

This Proposal is prepared based on the current schedule of rates for the year 2012 -13 and the total cost works out to.Rs.7367.00 Lakhs

S1. No	PACKAGE NOS.	NAME OF THE PACKAGE	PACKAGE AMOUNT IN LAKHS
1.	01/IAMWARM/WRD/ GML - River/ Works/V/ 2012 – 2013	Restoration and Regradation of Girudhumal river by construction of Additional vents to the link canal, Lining the Girudhumal Link Canal and Providing Scour vent to Palayanur tank weir to divert the Vaigai Flood water to the Girudhumal river in sivagangai district	1367.00
2.	02/IAMWARM/WRD/ GML - River/ Works/V/ 2012 – 2013	Restoration and regradation of Girudhumal river from Ambalathadi new anicut to Kattanur anicut LS 0 to 19400m in Madurai south taluk of Madurai district, Thirupuvanam Taluk of Sivagangai district and Thiruchuli Taluk of Virudhunagar District.	2400.00
3.	03/IAMWARM/WRD/ GML - River/ Works/V/ 2012 – 2013	Restoration and regradation of Girudhumal river from Kattanur anicut to Nallukuruchi anicut LS 19400 to 38500m in Thiruchuli Taluk of Virudhunagar District.	1700.00
4.	04/IAMWARM/WRD/ GML - River/ Works/V/ 2012 – 2013	Restoration and regradation of Girudhumal river from Nallukuruchi anicut to Regunathacauvery confluence Point LS 38500 to 66560m in Thiruchuli Taluk of Virudhunagar District and Kamuthi Taluk of Ramanathapuram District.	1900.00
		Sub Total	7367
		Environmental Cell	Nil

Under IAMWARM Project the following packages are proposed.

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	Grand Total	7367.00
	Ground Water	Nil
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Restoration and Regradation of Giruthumal river in Madurai, Sivagangai, Virudhunagar and Ramanathapuram district

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Sl. No	Description of work	Quant	ity	Amount in Lakhs	Remarks
a.	Earth Work Excavation in River Course	4012200	M <sup>3</sup>	1496.67	
b.	Earth Work for Bund Formation	1284835	M <sup>3</sup>	1209.90	
с.	Protection wall	1370	М	2228.59	
a.	Boundary Stone	1340	Nos	9.07	
b.	Head Sluice Repairs	4	Nos	112.28	
с.	Canal Lining	5600	М	1048.65	
a.	Scour Sluice	1	Nos	28.89	
b.	Repairs to Anicuts	3	Nos	79.66	
c.	Construction of Drop	2	Nos	86.84	
a.	Construction of New Anicut	4	Nos	840.90	
	Sub Total			7141.45	
	LS Provisions				
m.	Provisions for labour welfare fund, PS charges, contingencies, Advertisement charges, Documentation Charges, Name board, Photographic charges at 2.80%			225.55	
	Total			7367.00	

# WRO COST TABLE

Restoration and Regradation of Girudhumal river by construction of Additional vents to the link canal, Lining the Girudhumal Link Canal and Providing Scour vent to Palayanur tank weir to divert the Vaigai Flood water to the Girudhumal river in sivagangai district

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Sl. No	Description of work	Quantity	Amount in Lakhs	Remarks
I. Tank (	Component			
a.	Embankment improvements	269500Cum	139.58	
b.	Head sluice repair	4 Nos	112.28	
c.	Canal lining	5600m	1048.65	
d.	Scour sluice	1 No	28.89	
	Sub Total		1329.40	
	LS Provisions			
	Provisions for labour welfare fund, PS charges, contingencies, Advertisement charges, Documentation Charges, Name board, Photographic charges at 2.80%		37.60	
	Total		1367.00	

# PACKAGE- 01 / IAMWARM / WRD / GML-River / Works / V / 2012-2013 WRO COST TABLE

Restoration and regradation of Girudhumal river from Ambalathadi new anicut to Kattanur anicut LS 0 to 19400m in Madurai south taluk of Madurai district, Thirupuvanam Taluk of Sivagangai district and Thiruchuli Taluk of Virudhunagar District.

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Sl. No	Description of work	Quantity	Amount in Lakhs	Remarks
I. Tank (	Component			
a.	Earth Work Excavation in River Course	865900Cum	324.90	
b.	Earth Work for Bund Formation	363235 Cum	237.62	
c.	Protection Wall	920m	1509.43	
d.	Boundary Stone	388 Nos	2.63	
e.	Repairs to Odathur Anicut	1 No	5.00	
f.	Construction of New Anicut (Shankankulam) @ L.S 13100m	1 No	221.87	
g.	Gravel Toping	17100 Cum	22.62	
	Sub Total		2324.07	
	LS Provisions			
	Provisions for labour welfare fund, PS charges, contingencies, Advertisement charges, Documentation Charges, Name board, Photographic charges at 2.80%		75.93	
	Total		2400.00	

# PACKAGE- 02/IAMWARM/WRD/GML-River/Works/V/2012-2013 WRO COST TABLE

Restoration and regradation of Girudhumal river from Kattanur anicut to Nallukuruchi anicut LS 19400 to 38500m in Thiruchuli Taluk of Virudhunagar District

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Sl. No	Description of work	Quantity	Amount in Lakhs	Remarks
I. Tank (	Component			
a.	Earth Work Excavation in River Course	971000Cum	364.00	
b.	Earth Work for Bund Formation	310000 Cum	279.84	
c.	Protection Wall	450m	719.16	
d.	Boundary Stone	382 Nos	2.58	
e.	Repairs to Athikulam Anicut	1 No	5.50	
f.	Construction of Drop at LS 26100m	1 No	43.42	
	Construction of New Anicut to feed Varisaiyur Tank @ L.S 34930	1 No	210.22	
g.	Gravel Toping	16900 Cum	22.35	
	Sub Total		1647.07	
	LS Provisions			
	Provisions for labour welfare fund, PS charges, contingencies, Advertisement charges, Documentation Charges, Name board, Photographic charges at 2.80%		52.93	
	Total		1700.00	

# PACKAGE- 03/IAMWARM/WRD/GML-River/Works/V/2012-2013 WRO COST TABLE

Restoration and regradation of Girudhumal river from Nallukuruchi anicut to Regunathacauvery confluence Point LS 38500 to 66560m in Thiruchuli Taluk of Virudhunagar District and Kamuthi Taluk of Ramanathapuram District.

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Sl. No	Description of work	Quantity	Amount in Lakhs	Remarks
I. Tank (	Component			
a.	Earth Work Excavation in River Course	2042800Cum	764.30	
b.	Earth Work for Bund Formation	574600 Cum	518.69	
c.	Boundary Stone	570 Nos	3.86	
d.	Repairs to Nallukurichi Anicut	1 No	69.16	
e.	Construction of Drop at LS 38600m	1 No	43.42	
f.	Construction of New Anicut @ LS 44080m to feed keelaparithiyur tank and Anicut @ LS 48850m to Feed Kallikulam tank.	2 Nos	408.81	
g.	Gravel Toping	24700 Cum	32.67	
	Sub Total		1840.91	
	LS Provisions			
	Provisions for labour welfare fund, PS charges, contingencies, Advertisement charges, Documentation Charges, Name board, Photographic charges at 2.80%		59.09	
	Total		1900.00	

# PACKAGE- 04 / IAMWARM / WRD / GML-River / Works / V / 2012-2013 WRO COST TABLE

SI N Package No		Earth Work Excavation in River Course		Earth Work Earth Work Excavation in for Bund Prote River Course Formation		ction wall	Boundary Stone Rep		Head Sluice Canal Lining epairs		Scour Re Sluice A		Repairs to Anicuts		Construction of Drop		Constructi on of New Anicut		Sub Total	<u>LS</u> <u>Provisi</u> <u>ons</u>	Tota 1			
0		Qty	Amt	Qty	Amt	Qty	Amt	Qty	Amt	Qt y	Amt	Qty	Amt	Qt y	Amt	Qt y	Amt	Qty	Amt	Qt y	Amt		_	
1	Package No - I	1325 00	43.47	1370 00	96.11					4	112	5600	1048.65	1	28.89							1329.4	37.6	1367
2	Package No - II	8659 00	324.9	2632 35	260.24	920	1509.43	388	2.63							1	5			1	221.8 7	2324.07	75.93	2400
3	Package No - III	9710 00	364	3140 000	302.19	450	719.16	382	2.58							1	5.5	1	43.42	1	210.2 2	1647.07	52.93	1700
4	Package No - IV	2042 800	764.3	5746 00	551.36			570	3.86							1	69.16	1	43.42	2	408.8 1	1840.91	59.09	1900
	Total	4012 200	1496.6 7	1284 835	1209.9	1370	2228.59	1340	9.07	4	112	5600	1081.09	1	28.89	3	79.66	2	86.84	4	840.9	7141.45	225.55	7367

RESTORATION AND REGRADATION OF GIRIDHUMAL RIVER IN MADURAI, SIVAGANGAI, VIRUTHU NAGAR AND RAMANATHAPURAM DISTRICTS.

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Restoration and Regradation of Girudhumal river by construction of Additional vents to the link canal, Lining the Girudhumal Link Canal and Providing Scour vent to Palayanur tank weir to divert the Vaigai Flood water to the Girudhumal river in sivagangai district

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#### PACKAGE- 01/IAMWARM/WRD/GML-River/Works/V/2012-2013

Sl.No	Description	Qty	Unit	Cement in mt	Sand in m <sup>3</sup>	20mm Jelly in M3	40mm Jelly in M3	Rough stone	Gravel Filling	Steel	PVC Pipe 150mm dia`
1	M.10 Using 40mm HGB	40	M <sup>3</sup>	9	18		36				
2	M.15 Using 20mm	11559	M <sup>3</sup>	3621	5029	10058					
3	M.15 Using 60 % of 40mm & 40 % of 20 mm	1195	M <sup>3</sup>	387	538	430	645				
4	M.20 Using 20mm	80	M <sup>3</sup>	35	36	72					
5	R.C.C M20 using 20mm	55	M <sup>3</sup>	24	25						
6	Fabrication of steel	65	Qtl							65	
7	Rough Stone Dry Packing	565	Cum					565			
8	Gravel filling	26390	Cum						26390		
9	PVC Pipe 150mm dia	25700	m								25700
	Total			4075	5645	10560	681	565	26390	65	25700

#### **Requirment of materials**

Restoration and regradation of Girudhumal river from Ambalathadi new anicut to Kattanur anicut LS 0 to 19400m in Madurai south taluk of Madurai district, Thirupuvanam Taluk of Sivagangai district and Thiruchuli Taluk of Virudhunagar District.

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#### 20mm 40mm Sand in Cement Rough Gravel Sl.No Description Unit Jelly in Jelly in Steel Qty Filling in mt m<sup>3</sup> stone M3 M3 M.10 Using 40mm HGB М3 1 139 30 63 125 2 M.15 Using 20mm 368 $M^3$ 119 166 331 M.15 Using 60 % of 40mm & 40 % 3 46685 $M^3$ 15126 21008 16807 25210 of 20 mm M.20 Using 20mm 536 232 241 482 4 **M**<sup>3</sup> 5 Plastering 1:4 50 Sqm 0.0004 1.10 6 Fabrication 948 Qtl 948 7 Random Rubble 1:4 100 $M^3$ 0.0122 14 100 8 Rough Stone Dry Packing 2007 Cum 2007 Toal 15507 21492 17620 25335 100 2007 948

#### PACKAGE- 02/IAMWARM/WRD/GML-River/Works/V/2012-2013

## **Requirment of materials**

Restoration and regradation of Girudhumal river from Kattanur anicut to Nallukuruchi anicut LS 19400 to 38500m in Thiruchuli Taluk of Virudhunagar District

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Sl.No	Description	Qty	Unit	Cement in mt	Sand in m <sup>3</sup>	20mm Jelly in M3	40mm Jelly in M3	Rough stone	Gravel Filling	Steel
1	M.10 Using 40mm HGB	144	M <sup>3</sup>	31	65		130			
2	M.10 Using 60 % of 40mm & 40 % of 20 mm	1740	M <sup>3</sup>	376	783	626	940			
3	M.15 Using 20mm	1890	M <sup>3</sup>	61236	851	1701				
4	M.15 Using 40mm	1260	<b>M</b> <sup>3</sup>	40824	567		1134			
5	M.15 Using 60 % of 40mm & 40 % of 20 mm	21079	M <sup>3</sup>	6830	9486	7588	11383			
6	M.20 Using 20mm	487	M <sup>3</sup>	210	219	438				
7	M.25 Using 20mm	65	M <sup>3</sup>	30	29	59				
8	Fabrication	615	Qtl							615
9	Rough Stone Dry Packing	1747	Cum						1747	
	Toal			109537	11999	10413	13586	0	0	615

# PACKAGE- 03 / IAMWARM / WRD / GML-River / Works / V / 2012-2013 Requirment of materials

Restoration and regradation of Girudhumal river from Nallukuruchi anicut to Regunathacauvery confluence Point LS 38500 to 66560m in Thiruchuli Taluk of Virudhunagar District and Kamuthi Taluk of Ramanathapuram District.

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Sl.No	Description	Qty	Unit	Cement in mt	Sand in m <sup>3</sup>	20mm Jelly in M3	40mm Jelly in M3	Rough stone	Steel
1	M.10 Using 40mm HGB	382	M <sup>3</sup>	83	172		344		
2	M.15 Using 20mm	3480	<b>M</b> <sup>3</sup>	112752	1566	3132			
3	M.15 Using 40mm	1543	M <sup>3</sup>	49993	694		1389		
4	M.15 Using 60 % of 40mm & 40 % of 20 mm	3287	M <sup>3</sup>	1065	1479	1183	1775		
5	M.20 Using 20mm	357	<b>M</b> <sup>3</sup>	154	161	321			
6	M.25 Using 20mm	130	<b>M</b> <sup>3</sup>	60	59	117			
7	Fabrication	433	Qtl						433
8	Rough Stone Dry Packing	2580	Cum					2580	
	Toal			164107	4131	4754	3507	2580	433

# PACKAGE- 04 / IAMWARM / WRD / GML-River / Works / V / 2012-2013 Requirment of materials

#### Name of work :- Restoration and regradation of Girudhumal river by Construction of Additional Vents to the Link Canal , Lining the Girudhumal Link Canal and providing scour vents to the Konthagai tank weir and Palayanur tank Weir to divert the Vaigai Flood water to the Girudhumal river in Sivagangai District Package no :- 01/IAMWARM/WRD/GML - River/ Works/V/ 2012 - 2013.

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S1.No	Equipment	Numbers
1	Hydraulic Excavator (±0.90Cu.m)	8
2	Tippers/Lorries(8/10Tonne)	25
3	Power Rollers/Vibratory Power Rollers (including 2 power rollers of (± 0.90m width)	4
4	Water tankers (Truck mounted water tankers of ±10000 Litres)	4
5	Pneumatic Tampers/Earth Rammers (for compaction of earth fill adjoining the new concrete irrigation sluices to be constructed)	3
6	Air Compressors (±300cfm)	2
7	Plate Vibrators for compaction of sub grade and of bed bar concrete lining	3
8	Dozer (D6 or equivalent)	6
9	Mechanical Concrete mixers 14/10 cft, 10/7 cft	5
10	Concrete vibrators	7
11	Paver for bed and sides	2
12	Batching plant 12 – 15 m3/ Hour	1
13	Generator 45 KVA	2
14	Transit miller	4

#### **Requirment of Construction Equipments**

Restoration and regradation of Girudhumal river from Ambalathadi new anicut to Kattanur anicut LS 0 to 19400m in Madurai south taluk of Madurai district, Thirupuvanam Taluk of Sivagangai district and Thiruchuli Taluk of Virudhunagar District.

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Sl.No	Equipment	Numbers
1	Hydraulic Excavator (±0.90Cu.m)	10
2	Tippers/Lorries(8/10Tonne)	30
3	Power Rollers/Vibratory Power Rollers (including 2 power rollers of ( $\pm$ 0.90m width)	5
4	Water tankers (Truck mounted water tankers of ±10000 Litres)	10
5	Pneumatic Tampers/Earth Rammers (for compaction of earth fill adjoining the new concrete irrigation sluices to be constructed)	5
6	Air Compressors (±300cfm)	2
7	Plate Vibrators for compaction of sub grade and of bed bar concrete lining	5
8	Dozer (D6 or equivalent)	15
9	Mechanical Concrete mixers 14/10 cft, 10/7 cft	2
10	Concrete vibrators	1
11	Transit mixer	3
12	Concrete vibrators	13

# PACKAGE- 02 / IAMWARM / WRD / GML-River / Works / V / 2012-2013 Requirment of Construction Equipments

Restoration and regradation of Girudhumal river from Kattanur anicut to Nallukuruchi anicut LS 19400 to 38500m in Thiruchuli Taluk of Virudhunagar District

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Sl.No	Equipment	Numbers
1	Hydraulic Excavator (±0.90Cu.m)	10
2	Tippers/Lorries(8/10Tonne)	30
3	Power Rollers/Vibratory Power Rollers (including 2 power rollers of ( $\pm$ 0.90m width)	5
4	Water tankers (Truck mounted water tankers of ±10000 Litres)	10
5	Pneumatic Tampers/Earth Rammers (for compaction of earth fill adjoining the new concrete irrigation sluices to be constructed)	5
6	Air Compressors (±300cfm)	2
7	Plate Vibrators for compaction of sub grade and of bed bar concrete lining	5
8	Dozer (D6 or equivalent)	12
9	Mechanical Concrete mixers 14/10 cft, 10/7 cft	2
10	Concrete vibrators	15
11	Batching plant	1
12	Transit miller	3

# PACKAGE- 03 / IAMWARM / WRD / GML-River / Works / V / 2012-2013 Requirment of Construction Equipments

#### storation and regradation of Girudhumal river from Nallukuruchi anicut to Regunathacauvery confluence Point LS 38500 to 66560m in Thiruchuli Taluk of Virudhunagar District and Kamuthi Taluk of Ramanathapuram District.

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Sl.No	Equipment	Numbers
1	Hydraulic Excavator (±0.90Cu.m)	10
2	Tippers/Lorries(8/10Tonne)	30
3	Power Rollers/Vibratory Power Rollers (including 2 power rollers of ( $\pm$ 0.90m width)	5
4	Water tankers (Truck mounted water tankers of ±10000 Litres)	10
5	Pneumatic Tampers/Earth Rammers (for compaction of earth fill adjoining the new concrete irrigation sluices to be constructed)	5
6	Air Compressors (±300cfm)	2
7	Plate Vibrators for compaction of sub grade and of bed bar concrete lining	5
8	Dozer (D6 or equivalent)	15
9	Mechanical Concrete mixers 14/10 cft, 10/7 cft	5
10	Concrete vibrators	20
11	Batching plant	1
12	Transit miller	3

# PACKAGE- 04 / IAMWARM / WRD / GML-River / Works / V / 2012-2013 Requirment of Construction Equipments

Restoration and Regradation of Girudhumal river by construction of Additional vents to the link canal, Lining the Girudhumal Link Canal and Providing Sco Vaigai Flood water to the Girudhumal river in sivagangai district

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PACKAGE-01 / IAMWARM / WRD / GML-River / Works / V / 2012-2013

Construction methodology

Sl No	Description of Item							We	orking Mo	nths						
INO	-	Apr-13	May- 13	Jun-13	Jul-13	Aug-13	Sep-13	Oct-13	Nov-13	Dec-13	Jan-14	Feb-14	Mar-14	Apr-14	May-14	
								R	ainy Seaso	on						
	Earth work excavation															
1	Bund formation	10000	21600	21600	21600	21600	21600				21600	21600	21600	21600	21600	
2	Foundation	4200	4200	4200	4200	4200	4200				4200	4200	4200	4200	4500	Ī
	Concrete															
3	M.10 using 40mm			20	20											Ī
5	M15 using 60% 40mm and 40% of 20mm				200	200	200				200	200	195			

4	M 15 grade using 20mm		900	900	900	900	900		900	900	400	900	1000	
6	M 20 grade using 20mm			10	10	20	10		10	10	10			
7	R.C.C M 20 using 20mm											20	20	
8	Steel fabrication						10		20	10	10	15		
9	Rough stone												65	
10	Gravel filling	2000	2000	2000	2000	2300	2500		2500	2500	2500	2000	1000	
11	PVC pipe 150mm dia		2000	2000	2000	2000	2000		2000	2000	2000	2000	2000	

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Restoration and regradation of Girudhumal river from Ambalathadi new anicut to Kattanur anicut LS 0 to 19400m in Madurai south taluk of Madurai district and Thiruchuli Taluk of Virudhunagar District.

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PACKAGE-02 / IAMWARM / WRD / GML-River / Works / V / 2012-2013

Construction methodology

Sl No	Description of Item							Wo	rking Mon	ths					
INU		Apr-13	May-13	Jun-13	Jul-13	Aug-13	Sep-13	Oct-13	Nov-13	Dec-13	Jan-14	Feb-14	Mar-14	Apr-14	May-1
								R	ainy Seaso	on					
	Earth work excavation														
1	Bund	6000	6000	11000	14500	14500	14500				13500	20000	25000	25000	25000
2	Channel	21705	21705	39793	52455	52455	52455				48837	72500	72500	80000	80000
3	Foundation	1179	1179	2161	2848	2848	2848				2652	2848	3143	3143	3143
	Concrete														
4	M.10 using 40mm	50	50	39											
5	M 15 grade using 20mm	100	100	100	68										

6	M15 using 60% 40mm and 40% of 20mm	1000	3000	3000	3000	3000	3000		3000	3000	4000	4000	4000
7	M 20 grade using 20mm	16	16	29	38	38	38		35	38	42	52	52
8	Plastering			3	4	4	4		4	5	5	5	5
9	RR in CM 1:4			10	10	10	10		20	10	10	10	10

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Restoration and regradation of Girudhumal river from Kattanur anicut to Nallukuruchi anicut LS 19400 to 38500m in Thiruchuli Taluk of Virudhunagar Distr

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# PACKAGE-03 / IAMWARM / WRD / GML-River / Works / V / 2012-2013

Construction methodology

Sl No	Description of Item							Wo	rking Mon	ths					
110		Apr-13	May-13	Jun-13	Jul-13	Aug-13	Sep-13	Oct-13	Nov-13	Dec-13	Jan-14	Feb-14	Mar-14	Apr-14	May-14
								R	ainy Seaso	n					
	Earth work excavation														
1	Bund	10000	10000	20000	20000	20000	20000				25000	25000	30000	30000	20000
2	River	31730	31730	58171	76680	76680	76680					76680	84613	84613	84423
3	Foundation	468	468	859	1132	1132	1132					1280	1450	1450	1450
	Concrete														
4	M.10 using 40mm			50	50	44									
5	M 15 grade using 20mm			500	500	500	390								
6	M15 Using 40mm			68	89	126	126					100	120	120	120
7	M10 Using 60% of 40mm and 40% 20mm			123	174	144	123					140	140	150	150

8	M15 Using 60% of 40mm and 40% 20mm	 	1000	1000	1500	1500			2000	2500	2500	2000
9	M 20 grade using 20mm	 	26	48	48	34			40	45	45	45
10	M 25 grade using 20mm	 	5	7	5	5			6	6	6	6
11	Rough Stone Dry Packing	 		109	154	154			175	175	175	175

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# PACKAGE- 04 / IAMWARM / WRD / GML-River / Works / V / 2012-2013

Construction methodology

Sl	Description of Itom							V	Vorking M	onths						
No	Description of item	Apr-13	May-13	Jun-13	Jul-13	Aug-13	Sep-13	Oct- 13	Nov-13	Dec- 13	Jan-14	Feb-14	Mar-14	Apr-14	May-14	Ju
								R	ainy Seaso	on						
	Earth work excavation															
1	Bund	25000	25000	40000	40000	40000	40000				30000	40000	40000	40000	40000	40
2	River	75000	75000	120000	150000	150000	150000				150000	150000	150000	170000	150000	15
3	Foundation	285	285	523	690	690	690				350	350	800	800	900	8
	Concrete															
4	M.10 using 40mm			47	57	41	41				10	20	30	30	30	
5	M 15 grade using 20mm			280	369	369	369				100	100	300	300	200	
6	M15 Using 40mm			100	100	100	100				100	100	100	100	100	-
7	M15 Using 60% of 40mm and 40% 20mm			365	600	300	300				300	200	200	200	200	
8	M 20 grade using 20mm			19	40	40	20				20	18	30	30	30	
9	M 25 grade using 20mm			15	10	10	10				10	10	10	10	10	
10	Rough Stone Dry Packing				274	200	200				200	200	200	200	200	

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Road Level

1.5





Reach II







Hy	draulic Particulars :					
1.	Design Discharge	1000 cusees or 28.32 cumees				
2.	Free Board	0.75 m				
з,	Berm Width	1.13 m				
4.	Side Slope	1.5 H : 1 V				
		Reach I (LS 0 to 4060m)	Reach II (LS 4060m to 5490m)	Reach III (LS 5490m to 5600m)	Reach IV (LS 5600m to 9600m)	
5.	Bed Width	18m	18m	15m	15m	
6.	Bed Level	+119,500 m	+118.570 / +117.570 m	+117.140 m	+116.830 m	
7.	Bed Fall	1 in 4500	1 in 3325	1 in 355	1 in 1265	
8.	Full Supply Depth	1.50m	1.380m	0.96m	1,40m	
9.	Length of Canal	4060m	1430m	110m	4000m	
10.	Lining	PCC M15 Cast in situ	PCC M15 Cast in situ	Stone Pitched	Unlined	

Note :

4.00-

All dimensions are in metre unless otherwise specified.
This series of drawings should be supercede the series of drawings with this office Drg. No. 277/ 2012 to Drg. No. 275/ 2012 and Drg. No. 396/ 2012 to Drg. No. 508/ 2012.
This drawing should be read along with the Drg.No.416/ 2013 to Drg. No. 419 / 2013

	Government of Public Works I Office of the Superint Designs Circle, WRO, 0	Tamihadu Department tending Engineer, Chennai - 600 005.		
Designed & Drawn by Er. M.A. Saleena, AE	Aluta 10013	Name of work : Restoration and Regradation of Girudhamal river in Sivaganga, Vinudhunagar and Ramanathapuram		
Checked by Er. C. Sumathi, AEE	Alw 12013			
Verified by Er, K. Padmanabhan, EE (D) Er, V. Thiyagarajan, Dy. SE (D)	K-Padimanan	districts - Improvements to Girudhamal Link canal (Revised Design II)		
Recommended by Er. K.S.K Thulasiram, SE (D)	Bounderstaff	Typical Cross Section of Link Canal from LS 0m to LS 9600m		
Approved by Er.S. Anbazhagan, CE (DR&CS)	Show we.	Region: Circle:Lower Valgai Basin Madurai Region Circle, Sivagangai		
Drg. No. 416/2013	Scale 1 : 200	Approved by CE (DRACS) in Lr. No. 197 CE / SE(D / APE VII/ P 563-1/2012 dated 1 0 -01 -2013		

Sheet No. 1/4



#### General Notes:

- 1. The design have been done based on IS 10430-2000, IS 4558-1995, IS 3873 1993, IS 5256 1992, IS 13143 1991 and IS 3370(Part 1)-2009,
- 2. The Superintending Engineer, PWD, Lower Vaigai Basin Circle, WRO, Sivagangai vide Lr.No. 356m/DB/D03/F220/2012 dated 16.10.2012
  - has requested to increase the efficiency of the existing Girudhamal link canal.
- 3. Based on the details furnished by the Superintending Engineer, PWD, Lower Valgai Basin Circle, WRO, Sivagangai vide Lr.No. 356m/DB/D03/F220/2012 dated 16.10.2012 and the Executive Engineer, PWD, Saruganiyar Basin Division, WRO, Sivagangai vide Lr.No DB/2012/ dated.29.10.2012, the design has been formulated and approved by Chief Engineer(DR &CS) in Lr No: 264CE / SE(D)/ AEE VII / F.563-1 / 2012/ dt 16.11.2012.
- 4. As per the suggestions of Mr. R.K. Malhotra, Word Bank Consultant during the Preliminary meeting of the committee for "Assessment of left out works in IAMWARM sub basins" on 19.11.2012 the the design was revised and approved by Chief Engineer(DR &CS) in Lr No: 280CE / SE(D)/ AEE VII / F.563-1 / 2012/ dt 10.12.2012. Now it is also revised based on the suggestions of Mr. R.K. Malhotra, Word Bank Consultant during the meeting of the committee for "Assessment of left out works in IAMWARM sub basins" on 10.01.2013
- 5. The link canal has been designed for a Full Supply Discharge of 1000 cusecs (28.32 cumecs) .
- 6. As per the particulars furnished by the Executive Engineer, PWD, Saruganiyar Basin Division, WRO, Sivagangai it is assumed that the subgrade of the existing canal is made up of silt and clay having permeability between 10-4cm/sec to 10-6 cm/sec.
- 7. The design has been made on the assumption that the soil is of non expansive type. Before the execution , the swell pressure of the sample should be tested and if the swell pressure is found to be greater than 5t/m<sup>3</sup>, the revised design should be obtained from this office.
- 8. The entire length of the existing link canal is in cutting as the existing ground level is above the bed level of the canal.
- The side slope of the existing link canal has to be increased to 1V:1.5H from 1V:1H.
- 10. Free board of 0.75m is proposed.
- 11. It should be ensured that the relative density of sub grade at canal bottom and sides as 95% and 90% respectively.
- 12. Lining of canal is proposed with PCC M15 cast in situ from LS 0 to 4060m (Reach I) & LS 4060m to 5490m (Reach II) and with stone
- pitching from LS 5490m to LS 5600m (Reach III)
- 13. Concrete pavers is recommended for in situ lining.
- 14. Laying of insitu concrete lining should be done as per IS 3873-1993

15. It is suggested to provide two rows of longitudinal drains along the length of the canal at a spacing of 10.8m c/c and also transverse drains at every 10.8m

- 16. The drainage arrangements should be provided as per IS 4558:1995.
- 17. The longitudinal drains should be interconnected with the transverse drains.
- 18. The arrangement of Pressure Relief Valves are shown in the drawing.
- 19. Suitable transitions has to be provided between Reach II and Reach III where the bed width changes from 18m to 15m.
- 20. Expansion joints should be provided at intersection of every cross masonry structures along the length of the canal.
- 21. Contraction joints should be provided at 2.7mm c/c in both direction.
- 22. In no case the flow in the channel should overtop the free board.
- 23. All dimensions are in metre unless otherwise specified.
- 24. This series of drawings should be supercede the series of drawings with this office Drg. No. 272/ 2012
- to Drg. No. 275/ 2012 and Drg. No. 305/ 2012 to Drg. No. 309/ 2012.
- 25. This drawing should be read along with the Drg.No.416/2013 to Drg. No. 419/2013.



			Sheet No.4/4	
	Government of Public Works I Office of the Superint Designs Circle, WRO, C	Tamilnadu Department ending Engineer, Thennai - 600 00	5.	
Designed & Drawn by Er. M.A. Saleena, AE	61.000	Name of work:		
Checked by Er. C. Sumathi, AEE	10/01/2013	Restoration and Regradation of Gindhamaf liver in Sizangangai, Virudhunagar and Ramanethapuram Districts - Improvements of Girudhamal Link canal (Revised Design II)		
Verified by Er. K. Padmanabhan, EE (D) Er. V. Thiyagarajan, Dy. SE (D)	K-Padmanph Nol 113			
Recommended by Er. K.S.K Thulasiram, SE (D)	Energener	General Notes & Details of Pressur Relief Malves.		
Approved by Er.S. Anbazhagan, CE (DR&CS	84mm 10.1.19	Region: Madurai Region	Circle: Lower Vaigai Basin Circle, Sivagangai	

Not to Scale

Approved by CE (DR&C5) in Lr. No. 19 / AEE VII / F.563-1/2012 dated 10. 01 - 2013

CE/ SEC

Drg. No. 19/ 2013








2.1



3.11



## NOTES:

1. The analysis has been made based on IS 6966(Partl)-1989, IS11130-1984, IS6531-1984, IS12720-2004, IS 3370(Part 2)-2009, IS 456-2000 and CBIP publication No:179.

2. The Superintending Engineer, PWD, Lower Vaigai Basin Circle, WRO, Sivagangai vide Lr.No. 356m/DB/D03/F220/2012 dated 16.10.2012 has requested to provide additional vents to the existing Head Sluice of Girudhamal link canal. 3. Based on the details furnished by the Superintending Engineer, PWD, Lower Vaigai Basin Circle, WRO, Sivagangai vide Lr.No:356m/DB/D03/F220/2012 dated 16.10.2012 and the Executive Engineer, PWD, Saruganiyar Basin Division, WRO, Sivagangai vide Lr.No DB/2012/ dated.29.10.2012, the design has been formulated.

4. The additional vents - 2Nos has been designed for a maximum flood discharge of 395 cusecs or 11.20 cumecs with the overall discharge of 1000 cusecs or 28.32 cumecs.

5. The additional vents are proposed at the left side of the exisiting vents by converting the leftside abutment as pier and the leftside wing wall as divide wall as shown in the drawing.

6. The parameters of backfill material such as saturated unit weight and angle of internal friction have been assumed as 2t/cum and 22° respectively. Hence the soil of suitable type shall be used as backfill material.

7. The maximum stress below the abutment is  $21.67t/m^2$ . In the absence of bore hole details the foundation for abutment are proposed at a minimum depth of 0.9m below the protected bed. However during execution the bearing capacity shall be checked and ensured. The foundation shall be taken still lower if the sufficient bearing capacity is not available at the proposed level

8. The pier, abutment, wings walls, transition walls and return walls are proposed in PCC in M15. Surface reinforcement at the rate of 2.5 kg/m2 shall be provided in the abutment, wing walls, transition walls and returns in each direction ie. both horizontally and vertically. Spacing of such bars shall not exceed 200mm.

9. The bottom of the Downstream cutoff should not be keyed into the impervious layer as it will block the uplift pressure. If impervious layer is met with at the bottom of cutoff, suitable filter arrangements should be provided around the cutoff.

Necessary transitions may be provided at the downstream of stilling basin to negotiate the bed width and the side slope of canal as shown in the drawing.
All round cutoff is proposed below the transition walls for a thickness of 0.5m.

12. Transverse contraction joints with PVC water stops shall be provided at the locations separating the canal floor from the transition walls.

13. Weep holes with necessary filter arrangements should be provided in the abutment, upstream and downstream wingwalls and transition walls above the MWL and RWL respectively.

14. Minimum cover for wearing coat and other reinforcements shall be provided as 10cm and 5cm respectively.

15. The pier should be constructed monolithically with the apron floor.

16. This sheet should be read along with the series T.No.

265/2012 to T.No. 271 /2012

	and the second se		SHEET NO: 77			
OFFIC	GOVT. OF TAM PUBLIC WORKS E E OF THE SUPERINT S CIRCLE, WRO,	IILNADU DEPARTMENT ENDING ENGINER CHENNAI - 6000	ER, 005			
Designed & Drawn by Er. K. Sridevi, AE		Name of work :				
Checked by Er.C.Sumathi, AEE	QUU 11/2012	Restoration ar Girudhamal r	nd Regradation of iver in Sivaganga,			
rerified by k. Padmanabhan, EE (D) k. Padmanabhan, EE (D) k. Padmanabhan, EE (D) k. Padmanabhan, EE (D) k. Padmanabhan, EE (D)						
Er. V. Thiyagarajan, Dy. SE (D)		Sluice of Giru	dhamal Link Canal			
Recommended by Er. K. S.K.Thulasiram, SE (D)	Chancent	∠ Genera	l Notes			
Approved by Er.S. Anbazhagan, CE (DR&CS)	from the us	Region: Madurai Region	Circle : Lower Vaigai Basir Circle, Sivagangai			
T.NO:27// 2012	Not to Scale	Approved by CE (I SE(D) / AEE VII / 1	OR&CS) in Lr. No.264/ CF/ F.563-1/2012 dated /6.11.2012			
Er, C. Sumathi, AEE Verified by Er, K. Padmanabhan, EE (D) Er, V. Thiyagarajan, Dy. SE (D) Recommended by Er, K. S.K. Thulasiram, SE (D) Approved by Er, S. Anbazhagar, CE (DR&CS) T.NO:27/ 2012	Roman Control	Grudhamai r Virudhunaga districts - Providing adu Sluice of Giru Genera Region: Madural Region Approved by CE (I SE(D) / AEE VII / I	litiona dham l Notes Circl DR&CS F.563-1/			





#### OTES:

1. The analysis has been made based on IS 6966-1989, IS 12720-1993, IS 3370, IS 456 -2000 and IS 1893 - 2000.

2. The design has been formulated based on the particulars furnished by the EE, PWD, Saruganiyar Basin Division, Sivagangai, in Lr. No. DB/ 2002/ Dt: 29.10.2012.

3. The scour sluice has been designed for a maximum flood discharge of 415 cusecs or 11.75 cumecs, as requested by the EE, PWD, Saruganiyar Basin Division, Sivagangai.

4. The parameters of backfill material such as saturated unit weight and angle of internal friction have been assumed as 2t/cum and 30° respectively.

5. The depth of footing for abutment and wingwalls proposed below the sill levels are tentative and may be suitably modified according to the site conditions.

6. The proposed lengths of upstream and downstream returns are tentative and may be modified to suit the site conditions.

7. The apron floor, body wall, pier, abutment, wing walls and returns are proposed in M15 concrete using graded metal.

8. Surface reinforcement at the rate of 2.5Kg/m2 shall be provided in the abutment, wing wall, returns and pier in each direction ie both

horizontally and vertically. Spacing of such bars shall not exceed 200mm. 9. The bottom of the D/S cutoff should not be keyed into the impervious

layer as it will block the release of uplift pressure. As per the trial pit particulars furnished clayey strata starts from 0.15m below ground level and exists upto a depth of 1.85m below that. Hence sand filling for a depth of 0.5m is proposed below the floor replacing the existing clayey soil at the bottom of apron.

10. Weep holes with necessary filter arrangements should be provided in the upstream and downstream wingwalls above the FMWL and RWL respectively.

11. The maximum stress developed at the sill level of the abutment is 15.61t/sq.m. The SBC of the foundation media should be checked before taking up the work.

12. The design for the operating platform and the structural details of the shutters are to be finalised by P.W.W.S.

13. The top of operating platform has been fixed as +32.580m(TBL of tank) and the top of pier is fixed as +31.980m assuming that the depth of beam over the bed block is 0.6m. However it may vary as per the design obtained from P.W.W.S.

14. The pier should be constructed monolithic with the apron floor.

This sheet has to be read with the series of drawings from T No.<sup>324</sup>/2012 to T No.<sup>321</sup>/2012

		Sheet No. 3/3			
	Government Public Work Office of the Super Designs Circle, WRO	of Tamilnadu s Department intending Engineer, ), Chennai - 600 005			
Designed & Drawn by Er, K.Bhuvaneswari, AE - IV.	Stratter 12.12	Name of work :			
Checked by Er: P.Sasikala, AEE (VIII)	PSHAM	Construction of Scour Sluice for Palayanur Tank to Link Girudhuma			
Verified by Er. K. Padmanabhan, EE (D) Er, V. Thiyagarajan, Dy. SE (D)	K-Padmonal	River in Sivagangai District.			
Recommended by Fr. K.S.K.Thulasiram, SE (D)	Comenser	2 Designed drawing of : 124 GENERAL NOTES			
Approved by Br. 5. Anbazhagan, CE (DR&CS)	Sprintuin	Region: Madurai Circle:Lower Vaigai Basin Circle,Sivaganga			
Dro. No. 31 /2012.		Approved by CE (DRACS) in Lr. No. 2 "CE / SE(D) / Atili VIII / F54 A012 dated 14-12-12			



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# GRIDHUMAL RIVER - FLOW DIAGRAM (AMBALATHADI NEW ANICUT TO AMBALATHADI OLD ANICUT)



- 19 Parisanendal Tank
- 20 Naloor Tank







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SI.No.	Name of Tank	Ayacut (in Acres)			
1	Mela Vellore Tank	740 70			
2	Keela Vellore Tank	76.02			
3	Mangudi Tank	1038.45			
4	Moothakulam Tank	60.73			
5	Ambalathadi Tank	268.33			
6	Thaytharendal Tank	114.00			
7	Pottanalayam Tank	163.00			
8	Rangiam tank	1166.38			
g	Edirmarichan tank	86.86			
10	Karuvakkudi Tank	128.02			
11	S Nangoor Tank	152.02			
12	Peekulam Tank	87.28			
13	T Nedunkulam Tank	63.78			
14	Kalathur tank	13120			
15	Kallumadi Tank	88 37			
16	Naluvedankulam Tank	46.42			
17	Pallankulam Tank	22.38			
18	Kottakarai Tank	96.90			
19	Pulvaikarai Tank	222.21			
20	P Odaikulam Tank	59 30			
21	Vandavasi Tank	38.90			
22	Kalasthrivendal Tank	40.28			
23	Kundukulam Tank	138.65			
24	Singampuliyendal Tank	42.60			
25	Siruyanoor Tank	162.62			
26	Arunagiri Tank	74 14			
27	Saithanathanuram Tank	44 43			
28	Parisanendal Tank	38.30			
29	Naloor Tank	662.32			
30	V.Karisalkulam Tank	45.76			
31	Thamaraikulam Tank	203.75			
32	Thatchanenthal Tank	109.08			
33	T.Pulivankulam Tank	324.76			
34	Alathur Tank	166.57			
35	Poovalur Tank	75.60			
36	Thiruvidanallur Tank	115.04			
37	Kathaliendal Tank	100.41			
38	Rettaikulam Tank	162.90			
39	Senthanathi Tank	112.20			
40	Odathur Tank	276.30			
41	Eluvani Tank	245.60			
42	Vellarendal Tank	210.85			
43	Vellarendalchinna Tank	30.60			
44	Sengankulam Tank	254.44			
45	Kidakuli Tank	96.38			
46	Pirankulam Tank	18.92			
47	Puliankulam Tank	105.10			
48	Villakkanenthal Tank	103.26			



# GRIDHUMAL RIVER FROM LS 38500 to 66560 m



		20110
9.	Nallukurichi	96.12
10.	Valayankulam	8.37
11.	T.Kallikulam	56.27
12.	T.Punavasal	43.68
13.	A.Tharaikudi	142.34
14.	Abiramam	418.05
15.	Achankulam	44.3
16.	Muthathipuram	42.31

### PUBLIC WORKS DEPARTMENT WATER RESOURCES ORGANSATION

From Er. S.Anbazhagan, B.E., Chief Engineer, DR&CS, Chepauk, Chennai-600 005.

To The Chief Engineer, PWD Madurai Region, WRO, Madurai -1

### Lr.No. 299 CE(D)/SE(D) / AEE X / F 563 / 2012, dated 27.12.2012

Sir,

Sub: Restoration and Regradation of Gridhumal River in Sivagangai, Virudhunagar and Ramanathapuram Districts – Regradation of river – Revised cross section of formation of banks, protection wall and bank connections - Approved drawings communicated – regarding.

- Ref: 1. EIC, WRO, Ch 05 Lr no IAMWARM/Gridhumal river Regradation/2012 dt:20.09.2012 addressed to the CE, Madurai Region with copy to this office.
  - 2. SE(D) Inspection Notes Lr. No. 167 / AEE VII/ F 595 / dt 17.10.2012
  - SE, Vaippar Basin Circle, Virdhunagar Lr.No. 330 A/ M AE1/C-201/ Gridhumal / 2012 dt:15.10.12, received on 2.11.12.
  - 4. CE, DR&CS, WRO Ir.no.261 A/ AEE X / F563/ 2012 dated 12.11.2012
  - EE, Special Project Division, Madurai Ir.no. EE/ IAMWARM/ GML/ 2012 DT. 07.12.2012

With regard to the restoration and regradation of the Gridhumal river in Sivagangai, Virudhunagar and Ramanathapuram districts, the design for the regradation of the Gridhumal river from New Ambalathadi anicut to the confluence with Reghunathacauvery and protection wall were formulated, as per the directions given by the World bank Consultant Thiru. R.K.Malhotra and the copy of the approved drawings communicated vide letter 4<sup>th</sup>, cited. Since the soil parameters were not received from the Superintending Engineer, PWD, Vaippar Basin Circle, WRO, Virudhunagar, the soil parameters were assumed for the design.

Also during his visit from November 19-21, 2012, the World Bank Consultant had directed to provide necessary bank connections along the reaches of the Gridhamal river.

Further, in the reference 5<sup>th</sup> cited, the EE, Special Project Division, Madurai have furnished the above particulars, based on which the design has been revised and the revised drawings for

(5)

the banks, protection wall and the bank connections have been prepared and the copy of the approved drawings for the same is enclosed herewith for adoption.

Encl: Drawing - 4 Nos.

Sd/---dt.27.12,12 Chief Engineer, PWD, DR&CS, WRO, Chepauk, Chennal – 5.

Copy submitted to the Engineer-in-Chief, WRO, Chepauk, Chennai-5 Copy with enclosure to:

Copy with enclosure to: The Superintending Engineer, PWD, Vaippar Basin Circle, WRO, Virudhunagar The Executive Engineer, PWD, Upper Vaippar Basin Division, WRO, Rajapalayam The Executive Engineer, PWD, Vaippar Basin Division, WRO, Virudhunagar The Executive Engineer, PWD, Gundar Basin Division, WRO, Madurai The Executive Engineer, PWD, Special Project Division, WRO, Madurai The Executive Engineer, PWD, Special Project Division, WRO, Virudhunagar

The Superintending Engineer, PWD, Designs Circle, WRO, Chepauk, Chennai- 05. The Chief Engineer, DRCS - office file.

See for Chief Engineer, PWD, DR&CS, WRO, That Chepauk, Chennai – 5.



SI.No	I.No	REACH		BED LEVEL AT START OF REACH		F SLOPE	DESIGNED BED WIDTH	DISCHARGE	RGE DEPTH OF FLOW	N INSPECTION ROAD		SPOIL BANK		REMARKS		
	From		То	m						WIDTH	WIDTH	WIDTH	MINIMUM BERM WIDTH	4	1	
1	0 Km	10	1.86 Km	101.105	106,370	680	20	3600	1.986	4.00		3.00				
2	1.86 Km	to	6.90 Km	06.370	99,450	728	30	4660	1.884	4.00		3.00		Upto second Bridge		
3	6.90 Km	lo	9.10 Km	99.450	95.070	650	45	5880	1.656	4.00		3.00			-	
		-						1	1.690 -upto LS			-			1	
a.	9.10 Km	to	13.10 km	95.070	89.500	700	45	5880	1.690 to 2.180 - from LS 11100 to 13100m	4.00		3.00	-	Sangenkulam anicut		
5	13.10 Km	to	14.50 Km	88.500	86.775	820	50	6880	1.832	4.00		3.00				
6	14.50 Km	to	16.50 Km	86.775	85.080	1150	45	8270	2,417	4.00		3,00				
.7.	16.50 Km	to	17.00 km	85,080		950	45	8270	2.267	4.00		3.00				
Q	17.00 km	lo	18.00 km			950	45	8270	2.267	4.00	2.00	3.00	2.00			
0	18.00 km	to	19.40 Km		82.015	950	45	8270	2,267	4.00		3.00				
8	19.40 Km	to	26.10 Km	82.015	73.275	780	60	9620	1.981	4.00		3.00				
9	26.10 Km	ta	26.60 Km	72.475	71,860	813	65	9620	1.913	+.00	1.00	3.00		Drop@ 26.10km of bt.0.80m		
10	26.70 Km	(0	28.255 Km	71.860	87.175	330	65	9620	7:463	4.00	-	3.00				
11	28.255 Km	to	30.70 Km	67.175	64.850	1100	75	9620	1.925	4,00		3.00	1.44			
12	30.70 Km	to	32.15 km	64.850	63,560	1100	90	10650	1.837	4.00		3.00			0	
13	32.15 Km	to	34,93 Km	63.560	60.500	900	90	10650	2,750	4.00	2.00	4.00	2.00	Varisalyur anlaut	60.500	
14	34.93 Km	to	36.70 Km	60.300	59.700	295d	90	11300	(9 2.565	4.00	2,00	3.00	2.00		1/1000 22. 205	
15	36.70 Km	to	38.50 Km	59.700	55.400	545	90	11300	1.543	4.00		3.00			12200	
16	38.50 Km	to	43,225 km	54,900	50.9650	1200	60	11300	2.476	4.00	2.00	3.00	2.00	Drop at 38,60km of		
17	43.225 Km	to	44.08 Km	50.965	50,000	884	60	11300	2.475 © d/s of Veeracholon causeway & 2.980 © u/s of Keelaparuthiyor anicut	¥.00	2.00	\$.00	2.00	ht.1.50m Keela— paruthiyur anicut		
18	44.08 Km	to	44.70 Km	49.900	49.385	1200	60	11300	2.476	4.00	2,00	4.00	2.00			
19	44.70 Km	to	48,85 km	49,385	47.2000	1900	60	12530	2.850	4.00	3.00	4.00	3.00	Kallikulom Anicut		
	48.85 km		52.40 Km	45.200	43.710	1425	60	12530	2.790	4,00	2.00	4.00	2.00		1	his drawing supercedes the series of drawing
20	52,40 Km	to	53.07 Km	43.710	42.890	817	90	12530	1.852	4.00		3.00		-	T	is drawing should be read with the series of
21	53.07 Km	la	56.60 Km	42.890	38.960	900	120	13030	1.646	4.00		3.00			di	owings T No. 335/2012 to T No 338/2012 Sheet No 2
22	56.60 Km	10	58.15 Km	38.960	36.930	763	120	13530	1.602	4.00		3.00				GOVERNMENT OF TAME NADL
23	58.15 Km	to	59.30 Km	36.930	35.475	790	110	14030	1.742	4.00		3.00			OFFI	PUBLIC WORKS DEPARTMENT
24	59.30 Km	to	60,50 Km	35.475	34.190	933	110	14530	1.831	4.00		3,00		upto Causeway	Designed by	ESIGNS CIRCLE, WRO, CHENNAI - 600 005
25	50.50 Km	to	61.00 Km	34.190	33.660	943	110	14530	1.875	4.00		3.00			Er.V.Anvazhagon AE	31 27 min Name of work :
26	61.00 Km	to	63.70 Km	33,660	31.000	1015	70	15030	2.551	4.00	2.00	4.00	2.00		Er.S.Suganya AE Checked by	GRIDHUMAL RIVER IN SIVAGANGAI,
27	63.70 Km	to	65.00 Km	31.000	30.000	1300	75	15030	2.741	4.00	2.00	4,00	2.00		Er.E.Seena Divakaran AEE	2411-12 RAMANATHAPURAM DISTRICTS
28	65.00 Km	to	65.58 Km	30.000	28.000	780	60	16040	2.681	4.00	2.00	4.00	2.00		Er.K.Padmonobhan EE(D)	K Tadmint Destaned drawing of :
-					-										Recommended by	RESTORATION OF GRIDHAMAL RIVER
															Er.S.A. thurderom, SE(U) Approved by Er.S.Anbazhagon.CE.(DRACS) T.MO: 33.6 (2012)	Statist Magion: Circle: Periyar Velgol Medural Bosin Circle: Memoreo In CE(D.R.C.S) Lr NO395C/SE(D).

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Public Works Department Water Resources Organisation

From Er.S. Anbazhagan, B.E., Chief Engineer, PWD Design Research and Construction Support, WRO, Chennai 600 005. To The Chief Engineer, PWD, Madurai Region, WRO, Madurai.

#### Lr.No: 265 CE / AEE XI / F 563 / 2012 dated 16 .11.2012

Sir,

- Sub: Restoration and Regradation of Gridhumal river in Sivagangai, Virudhunagar and Ramanathapuram districts - Construction of an anicut across Gridhumal river to feed Sankankulam tank in Piramanur village of Manamadurai taluk in Sivagangai district- Design and Drawing of Anicut submitted for approval – reg.
- Ref: 1.EIC Lr. No. IAMWARM/Gridhumal river Regradation/2012 dt. 20.09.2012. addressed to the Chief Engineer, Madurai Region with copy to this office.

2.S.E., PWD, Vaippar Basin Circle, WRO, Virudhunagar Ir. No. 330A/M/ AEI/C-201/Gridhumal/2012/ dt. 15.10.12.

3.SE(D) Inspection notes Lr. No. / AEE VIII/F 595/ dt. 17.10.2012.

The Engineer-in-Chief in his letter cited has communicated a copy of the comments of Thiru. R.K. Malhotra, Consultant, World Bank, regarding the proposal for "Restoration and Regradatrion of Gridhumal river in Sivagangai, Virudhunagar and Ramanathapuram districts". As per his comments the following are to be designed by Chief Engineer, DR&CS.

1. The proposed river section including the embankments to be formed on the banks.

2. Provision of additional spillway at the head regulator of Link Canal and improvements to link canal.

3. The proposed check dams at of take points along the Gridhumal river.

Accordingly the site was inspected by the Superintending Engineer ( Designs) on

5.10.2012 along with SE Vaippar Basin Circle. The detailed project report was received from the SE, Vaippar Basin Circle vide reference 2nd cited. The various components as proposed by the SE Vaippar Basin Circle are as follows

1.Section for Gridhumal river from Ambalathadi new anicut to tail end of Gridhumal.

2. Anicut across Gridhumal river to feed Sankankulam tank @ LS 13100m

3. Anicut across Gridhumal riover to feed Keelaparithiyur tak @ LS 44100m

4. Protection wall for the narrow portion.

Based on the particulars received the bedslope, bed width, cross section and discharge of the Gridhumal river for various reaches was arrived and communicated to the field engineers. Accordingly the hydraulic particulars of the anicut across Gridhumal river to feed Sankankulam tank @ LS 13100m was arrived as

Bed slope	1	1 in 820
Bed width		80m
Maximum discharge	:	6880 cusecs

# MAXIMUM FLOOD DISCHARGE

The maximum flood discharge of the anicut has been arrived as 6880 Cusecs and the same is taken as design flood of the anicut.

# COMPUTATION OF REAR WATER LEVEL

From the L.S. of the river furnished, the bed slope of the river is computed as 1 in 820. The R.W.L. is computed for the regraded section of the river adopting this bed slope and works out to be +90.36m.

#### LAYOUT

It is proposed to provide 4 Nos. of scour vents (2 Nos. on either sides of anicut) of size  $1.50 \times 1.30$ m with sill level of +89.20m with a discharging capacity of 30.74 cumecs under maximum flood condition. Therefore, the anicut is proposed for a length of 70.00 m to dispose of the balance discharge. The total length of the structure considered for design in between the abutments is 80m.

### CREST LEVEL OF ANICUT

The crest level has been fixed at +90.50m. The average bed level of the river at the proposed site location is +89.50. Hence, the height of the anicut is 1.00m.

# COMPUTATION OF FRONT MAXIMUM WATER LEVEL

The front MWL for the above arrangements to dispose the design flood, adopting broad crested weir formula (having a  $C_d$  value of 0.577 for free weir condition), for the anicut is computed as +91.68m, with a head over crest of 1.18m.

#### DESIGN OF ANICUT PORTION

#### STABILITY OF BODY WALL

A broad crested weir having a top width of 1.00m, with vertical upstream face and downstream slope of 0.6:1 is proposed for the body wall portion. The stability of the body wall is checked for the following loading conditions,

1. Empty condition.

2. Water at MWL, and maximum tail water with uplift.

3. Water at crest level with full uplift, no tail water.

The stresses developed at the bottom of the body wall are computed and tabulated below.

SI.No	Description	Stress in t/m <sup>2</sup>	
		Maximum	Minimum
1.	Empty condition	2.96	0.94
2.	Water at MWL and maximum tail water	1.46	0.14
3.	Water at crest level with full uplift and no tail water	1.57	1.33

### DESIGN OF PERMEABLE FOUNDATION

Permeable foundation is proposed for the anicut. The apron floor is designed for the following flow conditions.

- 1. Subsurface flow condition
- 2. Surface flow condition

The thickness of the floor is arrived based on Khosla's theory for the subsurface flow condition adopting an exit gradient of 1 in 5. The energy dissipation arrangements such as basin level, basin length are arrived for the surface flow conditions. The apron floor and cut off are proposed in M15 concrete. Wearing coat to a thickness of 25cm is proposed in the stilling basin portion in M20 concrete.

### The salient features of the anicut are

Maximum Flood Discharge		194.82 Cumecs or 6880 Cusecs
Crest Level	1	+ 90.50m
Head over crest		1.18m

Front Maximum Water Level	-	Γ.	101.00	_
Rear Maximum Water Level			+91.68m	
Upstream bed level		:	+90.36m	
Downstream bed level		:	+89.50m	
Length of anique	_	ł	+88.50m	
Height of anicut		:	70.0m	-
		:	1.00m	-
Top width		:	1.00 m	-
Upstream slope		:	vertical	_
Downstream slope			0.6 : 1	_
Total length of floor			14.00m	_
Depth of upstream cutoff		+	3.3m	
Depth of downstream cutoff		+	2.20	
hickness of floor at toe		+	2.30m	
hickness of floor at end		1	J.80m	
ength of stilling basin		-	00m	
tilling Basin Level		8	3.02m	
Winchassen D	:	+	88.20m	-

Beyond the stilling basin 3 rows of CC blocks of size 1.5x1.5x0.9m in M15 concrete over an inverted filter of 60cm thickness are proposed. It is proposed to have a launching apron for a length of 6m having inner and outer thickness of 1.20m and

# DESIGN OF SCOUR VENT PORTION

# Computation of linear waterway

It is proposed to provide 4 vents of size 1.50 x 1.30m (2 nos. on each sides) to discharge 30.74 Cumecs or 1085.57 cusecs (i.e., 15.77% of the Maximum Flood Discharge) with sill level at +89.20m.

# Design of permeable foundation

The apron floor is designed similar to the anicut portion. The apron floor and cut off are proposed in M15 concrete. Wearing coat to a thickness of 25cm is proposed in the stilling basin portion in M20 concrete

Discharge through scour vent	:	30.74 Cumecs or 1085.57 Cusecs
Sill level	:	+89.20m
No. of vents	:	4 Nos.
Size of vent	:	1.50 x 1.30m
Total length of floor	:	22.00m
Upstream Bed Level	:	+89.20m
Downstream Bed level	:	+88.50m
Depth of upstream cutoff	:	3.00m
Depth of downstream cutoff	:	2.30m
Thickness of floor at toe	:	1.40m
Thickness of floor at end	:	1.00m
Stilling basin level	:	+87.70m
Length of stilling basin	:	10.25m

The salient features of the scour vent are as follows.

#### **Downstream Protection works**

Beyond the stilling basin 3 rows of CC blocks of size 1.5x1.5x0.9m in M15 concrete over an inverted filter of 60cm thickness are proposed. It is proposed to have a launching apron for a length of 6m having inner and outer thickness of 1.20m and 1.80m respectively.

# DESIGN OF PIER

The top of the pier is kept as +92.80 m. The height of the pier from the sill level is 3.60m. The stability of the pier is checked for the following conditions.

Condition1: All shutters are in closed condition	
--	--

Condition2: All shutters are in fully opened condition

Condition3: When one shutter closed and the adjacent being in opened condition.

It is suggested to provide a thickness of 1.0m and a length of 3.00m for the pier. The stresses developed due to the various forces involved at the four corners of the pier are computed for all the loading conditions and are summarized below. `

SI.No Condition		Stresses i	-		
		Corner A	Corner B	Corner C	Corner D
1	All shutters are in closed condition.	17.46	7.124	7.124	17.46
2	All shutters are in fully opened condition.	0.093	11.688	17.962	5.569
3	When one shutter closed and the adjacent being in opened condition	3.24	12.70	13.359	4.697

The pier is proposed in M15 concrete. Surface reinforcement at the rate of 2.5kg/m<sup>2</sup> shall be provided in each direction. ie both horizontally and vertically. Spacing of such bars shall not exceed 200m.

# DESIGN OF DIVIDE WALL

It is suggested to provide divide walls both at upstream and downstream side in between the scour vents and anicut portion. The proposed top levels of upstream and downstream divide wall are fixed as +90.50m. The divide walls are designed based on IS 12720 – 1993. The proposed thickness of the upstream and downstream divide walls are 0.45m and 0.60m respectively. The divide walls are proposed in M25 concrete. Necessary reinforcements in Fe415 are proposed for the moments developed.

# DESIGN OF ABUTMENT AND WING WALLS

The left and right abutments are designed for the earth pressure caused due to the backfill material. The top of operating platform is proposed at +92.80m. The parameters of the backfill material such as saturated unit weight and angle of internal friction have been assumed as 2.0t/m<sup>3</sup> and 30<sup>o</sup> respectively. Necessary wing walls have also been designed at the upstream and downstream sides.

The maximum stress developed at the sill level of the abutment is  $19.61t/m^2$ . The abutments and wing walls are proposed in M15 concrete using graded metal. It is suggested to provide surface reinforcement at the rate of 2.5 kg/m2 in the abutment wing walls and return walls at the water face in each direction. ie. both horizontally and vertically. Spacing of such bars shall not exceed 200 mm.

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#### DESIGN OF CANAL HEAD SLUICE

#### Computation of linear waterway

It is proposed to provide 3 vents of size  $1.00 \times 0.50$ m to draw the required discharge of 1.70 Cumecs or 60 Cusecs, with sill level at +89.80m.

#### Design of permeable foundation:

The apron floor is designed similar to the anicut portion. The apron floor and cut off are proposed in M15 concrete. Wearing coat to a thickness of 25cm is proposed in the stilling basin portion in M20 concrete

Discharge through Head Sluice vent	:	1.70 Cumecs or 60.00 Cusecs
Sill level	:	+89.80m
FSL in channel	:	+89.74m
No. of vents	:	3 Nos.
Size of vent	:	1.00 x 0.50m
Total length of floor	;	14.50m
Upstream Bed Level	:	+89.20m
Downstream Bed Level	:	+89.20m
Depth of upstream cutoff		2.00m
Depth of downstream cutoff	4	2.0m
Thickness of floor at toe	:	1.20m
Thickness of floor at end	:	1.00m
Stilling basin level	:	+88.80m
Length of stilling basin	:	5.80m

### The salient features of the Head Sluice are as follows

#### Design of breast wall

Since the computed FMFL of +91.68m is higher than the top of shutter +90.45, breast wall is proposed in the sluice vent. Necessary top and bottom beams are proposed in the breast wall. The thickness of the proposed breast wall is 0.20m. The sizes of top and bottom beams are  $0.2m \times 0.20m$  and  $0.25m \times 0.25m$  respectively. The breast wall and beams are proposed in M25 concrete.

#### **Downstream Protection works**

Beyond the stilling basin 1 row of CC block of size 1.5x1.5x0.9m in M15 concrete over an inverted filter of 60cm thickness is proposed. It is proposed to have a launching apron for a length of 2.0m having inner and outer thickness of 0.80m and 1.20m respectively.

#### DESIGN OF PIER

The top of the pier is kept as +92.80 m. The height of the pier from the sill level is 3.00m. The stability of the pier is checked for the following conditions.

Condition1: All shutters are in closed condition

Condition2: All shutters are in fully opened condition

Condition3: When one shutter closed and the adjacent being in opened condition.

It is suggested to provide a thickness of 0.90m and a length of 2.50m for the pier. The stresses developed due to the various forces involved at the four corners of the pier are computed for all the loading conditions and are summarized below.

SI.No	Condition	Stresses in t/m <sup>2</sup>					
		Corner A	Corner B	Corner C	Corner D		
1	All shutters are in closed condition.	19.409	1.976	1.976	19.409		
2	All shutters are in fully opened condition.	3.706	6.498	13,89	10.936		
3	When one shutter closed and the adjacent being in opened condition	12.446	4.568	4.778	12.818		

The pier is proposed in M15 concrete. Surface reinforcement at the rate of 2.5kg/m<sup>2</sup> shall be provided in each direction. ie both horizontally and vertically. Spacing of such bars shall not exceed 200m.

#### **Design of Abutment and Wing Walls**

The left and right abutments are designed for the earth pressure caused due to the backfill material. The top of operating platform is proposed at +92.80m and thus the top of the abutment is proposed at +92.80m. The parameters of the backfill material such as saturated unit weight and angle of internal friction have been assumed as  $2.0t/m^3$  and  $30^0$  respectively. Necessary wing walls are also designed at the upstream and downstream sides.

The maximum stress developed at the sill level of the abutment is  $15.20t/m^2$ . The abutments and wing walls are proposed in M15 concrete using graded metal. It is suggested to provide surface reinforcement at the rate of 2.5 kg/m2 in the abutment wing walls and return walls at the water face in each direction. ie. both horizontally and vertically. Spacing of such bars shall not exceed 200 mm.

# The following points should be considered before taking up the work.

- The bottom of the D/s cut off wall shall not be keyed into the impervious layer as it will block the release of uplift pressure. If impervious layer is met with at the bottom of D/s cutoff, suitable filter arrangements have to be provided around the cutoff wall.
- Weep holes with necessary filter arrangements shall be provided at the upstream and downstream wing walls above the M.W.L. and R.W.L. respectively.
- The depth of footing for abutment and wing walls proposed below the sill level is as per the details furnished.
- 4) Surface reinforcement at the rate of 2.5 kg/m<sup>2</sup> shall be provided in the abutment and wing walls at the water face in each direction. ie. Both horizontally and vertically. Spacing of such bars shall not exceed 200 mm.
- 5) Suitable Flood banks should be formed on the upstream and downstream side allowing a minimum free board of 1.0m above the water surface elevation as per the design given for Gridhumal river section.
- 6) The pier should be constructed monolithic with the apron floor.

Copies of the approved drawings of components of the anicut are sent herewith for necessary action.

Enclosure: Drawings - 8 Nos

-sd-Chief Engineer,PWD, DR&CS, WRO, Chennai -600 005. Copy to the Superintending Engineer, PWD, Designs Circle, WRO, Chennai 5. Enclosure: Plan-8 Nos

Copy to the Superintending Engineer, PWD, Vaippar Basin Circle, Virudhunagar Enclosure: Plan-8 Nos

Copy to the Executive Engineer, PWD, Special Project Division, WRO, Madurai. Enclosure: Plan-8 Nos

Spare copy to CE, DR&CS office file with enclosure.

Enclosure: Plan-8 Nos

11/12

for Chief Engineer,PWD, DR&CS, WRO, Chennai -600 005.





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#### NOTES:

1.The analysis has been made based on IS 6512-1984, IS 6966-1989, IS 12720-1993, IS 6531-1994, IS 13551-1992. IS 11130 - 1984 and IS 1893

- 2002.

2. The design has been formulated based on the particulars furnished by the SE, PWD, Vaippar Basin Circle, Virudhunagar, in Lr. No. 330A/M/ AE I/ C 201/Gridhumal/ Dt.15.10.12.

3. The anicut has been designed for a maximum flood discharge of 6880 cusecs.

4. The parameters of backfill material such as saturated unit weight and angle of internal friction have been assumed as 2t/cum and 30° respectively.

5. The depth of footing for abutment and wingwalls proposed below the sill levels are tentative and may be suitably modified according to the site conditions.

6. The proposed lengths of upstream and downstream returns are tentative and may be madified to suit the site conditions.

7. The apron floor, body wall, pier, abutment, wing walls and returns are proposed in M15 concrete using graded metal.

8. Surface reinforcement at the rate of 2.5Kg/m2 shall be provided in the abutment, wing wall, returns and pier in each direction ie both horizontally and vertically. Spacing of such bars shall not exceed 200mm.

9. Weep holes with necessary filter arrangements should be provided in the upstream and downstream wingwalls above the FMWL and RWL respectively.

10. Suitable Flood banks should be formed on the upstream and dowstream side allowing a minimum free board of 1.0m above the water surface elevation.

11. The maximum stress developed at the sill level of the downstream wing wall of anicut is 19.61t/sq.m. The SBC of the foundation media should be checked before taking up the work.

12. The downstream protection works should be maintained periodically for effective functioning of the anicut.

13. The design for the operating platform and the structural details of the shutters are to be finalised by P.W.W.S.

11. T a top of operating platform has been tentatively fixed as +92.80m assuming that the depth of beam and the thickness of chequered plates over the bed block is 0.40m. However it may vary as per the design obtained from P.W.W.S.

15. The scour vents shall be in fully opened condition during floods.

16. The pier should be constructed monolithic with the apron floor.

17. Transverse contraction joints with PVC water stops shall be provided in the anicut portion at every 25m interval and at the locations specified in the drawing.

18. U/s divide wall may be constructed for a length as shown in the drawing.

19. D/s divide wall is to be constructed monolithic with the D/s apron floor upto the end of stilling basin of scour vent portion.

20. A clear cover of 75mm shall be provided for the reinforcement provided in the stilling basin portion of anicut, scour vent and canal sluice.

21. The bed level of river, at the D/S of the anicut portion should be regraded from the proposed bed level of +88.50 m to a slope of 1 in 820 for the river width of 80m atleast for a distance of 500m.

22. The bed level of the river at the scour vent portion shall be maintained at a level of +89.20m so as to enable effective flow through scour vent.

23. The canal sluice has been designed for a discharge of 60 cusecs. 24. The top and bottom beams of breast wall in canal sluice should be

embedded into the pier for a depth of 0.3m on both sides. 25. Necessary transitions shall be made at the end of the canal sluice

portion to negotiate the width of the canal sluice with the bed width of the canal.

This drawing has to be read with the series of drawings T No 23/2012 to T No 283/2012

Sheet No 8/8

OFFICE OF THE DESIGNS C	SUPERINTEND	EPARTMEI	NT NEER, PWD, 600 005	
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Public Works Department Water Resources Organisation

From **Er.S. Anbazhagan**, B.E., Chief Engineer, PWD Design Research and Construction Support, WRO, Chennai 600 005. To The Chief Engineer, PWD, Madurai Region, WRO, Thallakulam, Madurai + 625 002.

#### Lr. No. 266 CE / SE (D) / AEE III / F 563 / 2012 dated 16.11.2012

#### Sir,

Sub: Restoration and Regradation of Gridhumal river in Sivagangai, Virudhunagar and Ramanathapuram districts – Construction of an anicut across Gridhumal river to feed Keelaparithiyur tank at LS 44080m– Design and Drawing of Anicut and Headsluice – Copy of approved drawings - communicated – regarding.

Ref: 1.EIC Lr. No. IAMWARM/Gridhumal river Regradation/2012 dt. 20.09.2012, addressed to the Chief Engineer, Madurai Region with copy to this office. 2.S.E., PWD, Vaippar Basin Circle, WRO, Virudhunagar Ir. No. 330A/M/ AEI/C-201/Gridhumal/2012/ dt. 15.10.12.

3.SE(D) Inspection notes Lr. No. / AEE VIII/F 595/ dt. 17.10.2012.

The Engineer-in-Chief in his letter cited has communicated a copy of the comments of Thiru. R.K. Malhotra, Consultant, World Bank regarding the proposal for "Restoration and Re-gradation of Girdhumal river in Sivagangai, Virudhunagar and Ramanathapuram districts". As per his comments the following are to be designed by Chief Engineer, DR&CS.

- The proposed river section including the embankments to be formed on the banks.
- Provision of additional spillway at the head regulator of Link Canal and improvements to Link canal.

3. The proposed check dams at off take points along the Gridhumal river.

Accordingly the site was inspected by the Superintending Engineer (Designs) on 5.10.2012 along with the SE, Vaippar Basin Circle, Virudhunagar. The detailed project report was received from the SE, Vaippar Basin Circle vide reference 2nd cited. The various components as proposed by the SE Vaippar Basin Circle are as follows

1. Section for Gridhumal river from Ambalathadl new anicut to tail end of Gridhumal.

2. Anicut across Gridhumal river to feed Sankankulam tank @ LS 13100m

3. Anicut across Gridhumal river to feed Keelaparithiyur tank @ LS 44080m

4. Protection wall for the narrow portion.

Based on the particulars received the bedslope, bed width, cross section and discharge of the Gridhumal river for various reaches was arrived and communicated to the field engineers. Accordingly the hydraulic particulars of the anicut across Gridhumal river to feed Keelaparithiyur tank @ LS 44080m was arrived as

Bed slope	¥	1 in 1200
Bed width	:	60m
Maximum discharge	+	11300 cusecs

#### MAXIMUM FLOOD DISCHARGE

The maximum flood discharge of the anicut has been arrived as 11300 Cusecs and the same is taken as design flood of the anicut.

#### COMPUTATION OF REAR WATER LEVEL

From the L.S. of the river furnished, the bed slope of the river is computed as 1 in 1200. The R.W.L. is computed for the regraded section of the river adopting this bed slope and works out to be +52.40m.

#### LAYOUT

It is proposed to provide 4 Nos. of scour vents (2 Nos. on either sides of anicut) of size  $1.20 \times 0.60$ m with sill level of +50.00m with a discharging capacity of 34.34 cumecs under maximum flood condition. Therefore, the anicut is proposed for a length of 51.60 m to dispose of the balance discharge. The total length of the structure considered for design in between the abutments is 60m.

#### CREST LEVEL OF ANICUT

The crest level has been fixed at +50.60m. The average bed level of the river at the proposed site location is +50.00m. Hence, the height of the anicut is 0.60m.

#### COMPUTATION OF FRONT MAXIMUM WATER LEVEL

The front MWL for the above arrangements to dispose the design flood, adopting narrow crested weir formula (having a  $C_d$  value of 0.625 for free weir condition), for the anicut is computed as +52.97m, with a head over crest of 2.37m.

### DESIGN OF ANICUT PORTION

### STABILITY OF BODY WALL

A broad crested weir having a top width of 1.20m, with vertical upstream face and downstream slope of 0.75:1 is proposed for the body wall portion. The stability of the body wall is checked for the following loading conditions,

1. Empty condition.

- 2. Water at MWL, and maximum tail water with uplift.
- 3. Water at crest level with full uplift, no tail water.

The stresses developed at the bottom of the body wall are computed and tabulated below:

SI.No	Description	Stress in t/m <sup>2</sup>		
		Maximum	Minimum	
1.	Empty condition	1.72	0.76	
2.	Water at MWL and maximum tail water	0.43	0.01	
3.	Water at crest level with full uplift and no tail water	1.043	0.845	

#### DESIGN OF PERMEABLE FOUNDATION

Permeable foundation is proposed for the anicut. The apron floor is designed for the following flow conditions.

- 1. Subsurface flow condition
- 2. Surface flow condition

The thickness of the floor is arrived based on Khosla's theory for the subsurface flow condition adopting an exit gradient of 1 in 5. The energy dissipation arrangements such as basin level, basin length are arrived for the surface flow conditions. The apron floor and cut off are proposed in M15 concrete. Wearing coat to a thickness of 25cm is proposed in the stilling basin portion in M20 concrete.

The salient features of the anicut are

Maximum Flood Discharge	1	319.977 Cumecs or 11300 Cusecs		
Crest Level	;	+ 50.60m		
Head over crest	:	2.37m		
Front Maximum Water Level	:	+52.97m		
Rear Maximum Water Level	:	+52.40m		
Upstream bed level	;	+50.00m		
Downstream bed level	1	+49.90m		
Length of anicut	:	51.60m		
Height of anicut	:	0.60m		
Top width	:	1.20 m		
Upstream slope	1	vertical		
Downstream slope	:	0.75:1		
Total length of floor		15.00m		
Depth of upstream cutoff		1.6m		
Depth of downstream cutoff	*	2.80m		
Thickness of floor at toe	1	: 0.60m		
Thickness of floor at end		0.90m		
Length of stilling basin		9.17m		
Stilling Basin Level	:	+49.50m		

## DESIGN OF SCOUR VENT PORTION

#### Computation of linear waterway

It is proposed to provide 4 vents of size 1.20 x 0.60m (2 nos. on each sides) to discharge 34.34 Cumecs or 1212.72 cusecs (i.e., 10.732% of the Maximum Flood Discharge) with sill level at +50.00m.

#### Design of permeable foundation

The apron floor is designed similar to the anicut portion. The apron floor and cut off are proposed in M15 concrete. Wearing coat to a thickness of 25cm is proposed in the stilling basin portion in M20 concrete

Discharge through scour vent	:	34.34 Cumecs or 1212.72 Cusecs
Sill level	:	+50.00m
No. of vents	1	4 Nos.
Size of vent		1,20 x 0.60m
Total length of floor	1	15.00m
Upstream Bed Level	:	+50.00m
Downstream Bed level	:	+49.90m
Depth of upstream cutoff	:	1,60m
Depth of downstream cutoff	:	2.80m
Thickness of floor at toe	;	1.10m
Thickness of floor at end	:	0.60m
Stilling basin level	:	+49.50m
Length of stilling basin	:	9.00m

The salient features of the scour vent are as follows.

#### **PROTECTION WORKS:**

#### **Upstream Protection works**

It is proposed to have a bed pitching of 0.60 m thickness for a length of 3 m in the upstream side.

#### **Downstream Protection works**

Beyond the stilling basin 5 rows of CC blocks of size 1.5x1.5x0.9m in M15 concrete over an inverted filter of 60cm thickness are proposed. It is proposed to have a launching apron for a length of 8m having inner and outer thickness of 1.40m and 2.10m respectively.

#### DESIGN OF PIER

The top of the pier is kept as +53.97 m. The height of the pier from the sill level is 3.97m. The stability of the pier is checked for the following conditions.

Condition1: All shutters are in closed condition

Condition2: All shutters are in fully opened condition

Condition3: When one shutter closed and the adjacent being in opened condition.

It is suggested to provide a thickness of 0.90m and a length of 3.00m for the pier. The stresses developed due to the various forces involved at the four corners of the pier are computed for all the loading conditions and are summarized below.

SI. No.	Condition	Stresses in t/m <sup>2</sup>				
		Corner A	Corner B	Corner C	Corner D	
1	All shutters are in closed condition.	8.847	16.156	16.156	8.847	
2	All shutters are in fully opened condition.	0.218	15.025	20.015	5.208	
3	When one shutter closed and the adjacent being in opened condition	-0.833	17.87	18.561	1.672	

The pier is proposed in M15 concrete. Surface reinforcement at the rate of 2.5kg/m<sup>2</sup> shall be provided in each direction, ie both horizontally and vertically. Spacing of such bars shall not exceed 200m.

### DESIGN OF ABUTMENT AND WING WALLS

The left and right abutments are designed for the earth pressure caused due to the backfill material. The top of operating platform is proposed at +53.97m. The parameters of the backfill material such as saturated unit weight and angle of internal friction have been assumed as  $2.0t/m^3$  and  $30^0$  respectively. Necessary wing walls have also been designed at the upstream and downstream sides.

The maximum stress developed at the sill level of the abutment is 20.537t/m<sup>2</sup>. The abutments and wing walls are proposed in M15 concrete using graded metal. It is suggested to provide surface reinforcement at the rate of 2.5 kg/m2 in the abutment wing walls and return walls at the water face in each direction. ie. both horizontally and vertically, Spacing of such bars shall not exceed 200 mm.

#### DESIGN OF CANAL HEAD SLUICE

#### Computation of linear waterway

It is proposed to provide 4 vents of size 0.90 x 0.40m to draw the required discharge of 1.70 Cumecs or 60 Cusecs, with sill level at +50.20m.

#### Design of permeable foundation:

The apron floor is designed similar to the anicut portion. The apron floor and cut off are proposed in M15 concrete. Wearing coat to a thickness of 25cm is proposed in the stilling basin portion in M20 concrete

Discharge through Head Sluice vent	1	1.70 Cumecs or 60.00 Cusecs
Sill level	3	+50.20m
FSL in channel	:	+50.40m
No. of vents	1	4 Nos.
Size of vent	:	0.90 x 0.40m
Total length of floor	:	15.00m
Upstream Bed Level	1	+50.00m
Downstream Bed Level	:	+49.87m
Depth of upstream cutoff	:	1.60m
Depth of downstream cutoff	4	2.80m
Thickness of floor at toe	:	1.60m
Thickness of floor at end	:	1.20m
Stilling basin level		+49.57m
Length of stilling basin	1:	9.31m

#### The salient features of the Head Sluice are as follows

#### Design of breast wall

Since the computed FMFL of +52.97m is higher than the top of shutter +50.75, breast wall is proposed in the sluice vent. Necessary top and bottom beams are proposed in the breast wall. The thickness of the proposed breast wall is 0.15m. The size of bottom beam is 0.25m x 0.25m. The breast wall and beams are proposed in M25 concrete.

#### **Upstream Protection works**

It is proposed to have a bed pitching of 0.60 m thickness for a length of 3 m in the upstream side.

#### **Downstream Protection works**

Beyond the stilling basin 1 row of CC block of size 1.5x1.5x0.9m in M15 concrete over an inverted filter of 60cm thickness is proposed. It is proposed to have a launching apron for a length of 1.50m having inner and outer thickness of 0.70m and 1.10m respectively.

#### DESIGN OF PIER

The top of the pier is kept as +53.47 m. The height of the pier from the sill level is 3.47m. The stability of the pier is checked for the following conditions.

Condition1: All shutters are in closed condition

Condition2: All shutters are in fully opened condition

Condition3: When one shutter closed and the adjacent being in opened condition.

It is suggested to provide a thickness of 0.90m and a length of 3.00m for the pier. The stresses developed due to the various forces involved at the four corners of the pier are computed for all the loading conditions and are summarized below.

SI. No.	Condition	Stresses in t/m <sup>2</sup>				
		Corner A	Corner B	Corner C	Corner D	
1	All shutters are in closed condition.	7.544	9.225	9.225	7.544	
2	All shutters are in fully opened condition.	12.846	0.878	5.890	17.858	
3	When one shutter closed and the adjacent being in opened condition	11.944	6.079	4.421	10.286	

The pier is proposed in M15 concrete. Surface reinforcement at the rate of 2.5kg/m<sup>2</sup> shall be provided in each direction. ie both horizontally and vertically. Spacing of such bars shall not exceed 200m.

#### **Design of Abutment and Wing Walls**

The left and right abutments are designed for the earth pressure caused due to the backfill material. The top of operating platform is proposed at +53.97m and thus the top of the abutment is proposed at +53.97m. The parameters of the backfill material such as saturated unit weight and angle of internal friction have been assumed as  $2.0t/m^3$  and  $30^0$  respectively. Necessary wing walls are also designed at the upstream and downstream sides.

The maximum stress developed at the sill level of the abutment is  $15.87t/m^2$ . The abutments and wing walls are proposed in M15 concrete using graded metal. It is suggested to provide surface reinforcement at the rate of 2.5 kg/m2 in the abutment wing walls and return walls at the water face in each direction. ie. both horizontally and vertically. Spacing of such bars shall not exceed 200 mm.

# following points should be considered before taking up the work.

- The bottom of the D/s cut off wall shall not be keyed into the impervious layer as it will block the release of uplift pressure. If impervious layer is met with at the bottom of D/s cutoff, suitable filter arrangements have to be provided around the cutoff wall.
- Weep holes with necessary filter arrangements shall be provided at the upstream and downstream wing walls above the M.W.L. and R.W.L. respectively.
- The depth of footing for abutment and wing walls proposed below the sill level is as per the details furnished.

- 4) Surface reinforcement at the rate of 2.5 kg/m<sup>2</sup> shall be provided in the abutment and wing walls at the water face in each direction. ie. Both horizontally and vertically. Spacing of such bars shall not exceed 200 mm.
- Suitable Flood banks should be formed on the upstream and downstream side allowing a minimum free board of 1.0m above the water surface elevation.
- 6) The pier should be constructed monolithic with the apron floor.

Copy of approved drawings of the anicut and head sluice are enclosed for adoption. Encl: Drawings - 9 Nos (Drawing Nos. 284 / 2012 to 292 / 2012)

> Sd /- 16.11.2012 Chief Engineer, PWD, DR&CS, WRO, Chennai - 5.

Copy to the Superintending Engineer, PWD, Vaippar Basin Circle, WRO, Virudhunagar for information along with the enclosures.

Copy to the Superintending Engineer, PWD, Designs Circle, WRO, Chennai-5 along with the enclosures.

LCOPY to the Executive Engineer, PWD, Special Project Division, WRO, Madurai along with the enclosures.

Spare copy to CE, DR&CS office file along with the enclosures.

Spare copy to AEE III along with the enclosures.

for Chief Engineer, PWD, DR&CS, WRO, Chennai - 5.









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#### General Notes:

I. Anicut

- The analysis has been made based on IS 6512-1984, IS 6966-1989, IS 12720-1993, IS 6531-1994, IS 1893-2002, IS 3370 - 2009 and IRC 6-2000.
- The design has been formulated based on the particulars furnished by the the Superintending Engineer, PWD, Vaippar Basin Circle, WRO, Virudhunagar in Lr. No. 330A / M / AE1 / C-201/ Gridhumal / 2012 dated 15:10:2012.
- 3. The anicut has been designed for a maximum flood discharge of 11,300 cusecs (320 cumecs).
- 4. The parameters of backfill material such as saturated unit weight and angle of internal friction have been assumed as 2 t/cu.m. and 22° respectively. Hence suitable backfill material may be used accordingly.
- The depth of footing for abutment and wingwalls proposed below the sill levels are tentative and may be suitably modified according to the site conditions.
- The proposed lengths of upstream and downstream returns are tentative and may be modified to suit the site conditions.
- The apron floor, body wall, cut-off walls, pier, abutment, wingwalls and returns are proposed in M15 grade of concrete.
- Surface reinforcement at the rate of 2.5 kg/sq.m. shall be provided in the pier, abutment, wing walls and returns at water face in each direction i.e., both horizontally and vertically. Spacing of such bars shall not exceed 200mm.
- 9. The bottom of the D/s cutoff should not be keyed into the impervious layer as it will block the release of uplift pressure. If impervious layer is met with at the bottom of D/s cutoff, suitable filter arrangements should be provided around the cutoff.
- Weep holes with necessary filter arrangements should be provided in the upstream and downstream wingwalls above the FMWL and RWL respectively.
- 11. Suitable Flood banks should be formed allowing a minimum free board of 1.0m above the water surface elevation in the upstream side as given in the tabulation in sheet no. 8/9 and above RWL in the downstream side.
- 12. The maximum stress developed at the bottom of the D/s Wing wall @ start i.e., @ +49,500 m level is 205.68 kN/sq.m. The SBC of the foundation media should be checked before taking up the work.
- The proposed downstream protection works should be maintained periodically for effective functioning of the anicut.
- 14. The design for the operating platform and the structural details of the shutters are to be finalised by the Public Works Workshop.
- 15. The top of operating platform has been tentatively fixed as +54,470 m assuming that the depth of beam and the thickness of chequered plates over the bed block is 0.50 m. However it may vary as per the design obtained from P.W.W.S. If there is any major variation, it shall be referred to Designs Circle for revision.
- 16. The Pier should be constructed monolithic with apron floor.
- 17. Transverse contraction joints with PVC water stops shall be provided along the entire cross section of the articut at one location i.e., at the centre of anicut length and at the locations specified in the drawing separating the apron floor from the wing walls and abutment.
- 18. The bed level of the river at the D/s of the anicut should be regraded from the proposed D/s bed level of +49.900 m to a slope of 1 in 1200 for a width of 60 m atleast for a distance of 500 m.
- 19. The scour vents should be in fully opened condition during floods.
- 20. A clear cover of 75 mm shall be provided for the reinforcement provided in the stilling basin portion.
- The structure is designed as Permeable foundation. During execution, if impervious strata found, the Design Circle shall be referred for revised design.

#### II. Head Sluice

- 1. The Head shuice has been designed for a discharge of 60 cusecs (1.70 cumeos).
- 2. Four number of vents of size 0.90 m x 0.40 m is proposed for the head sluice portion.
- 3. The energy dissipation arrangements for the head sluice portion have been designed for a discharge of 60 cusecs. Therefore the vent opening should be so adjusted to maintain the discharge of 60 cusecs in the canal by keeping the Full Supply Depth (FSD) of 0.53 m at Maximum Flood Condition.
- Necessary transition may be provided at the end of the head sluice portion to negotiate the bed width of canal and the side slope.
- 5. The bottom of the D/s cutoff should not be keyed into the impervious layer as it will block the release of uplift pressure. If impervious layer is met with at the bottom of D/s cutoff, suitable filter arrangements should be provided around the cutoff.
- Weep holes with necessary filter arrangements should be provided in the upstream and downstream wingwalls above the FMWL and FSL respectively.
- The proposed downstream protection works should be maintained periodically for effective functioning of the head sluice.
- The proposed lengths of upstream and downsteam returns are tentative and may be modified to suit the site conditions.
- The maximum stress developed at the bottom of the U/s Wing wall i.e., @+49.570 m level is 179.82 kN/sq.m. The SBC of the foundation media should be checked before taking up the work.
- 10. The Pier should be constructed monolithic with the apron floor.
- The Top and bottom beam of breast wall shall be embedded into the pier for a depth of 0.30 m on both sides.
- 12. The top of operating platform has been tentatively fixed as +53,970 m assuming that the depth of beam and the thickness of chequered plates over the bed block is 0.50 m. However it may vary as per the design obtained from Public Works Workshop. If there is any major variation it shall be referred to Designs Circle for revision.
- 13. This sheet should be read along with the Drg. no. 284 / 2012 to Drg. no. 292/2012.

		-	Sheet No. 9/9	
C De	Government of Public Works 1 Office of the Superint signs Circle, WRO, 0	Tamilnadu Department tending Engines Chennai - 600	er, D05.	
Designed & Drawn by Er. P. Karthikeyan, AB - III	PKill winh2	Name of work	10.000	
Checked by Er. G.R. Suganthi, AEE - III	Arguilter	Girdhumal ri	nd Regradation of ver - Construction of Girudhumal river to feed	
Verified by Er. K. Padmanabhan, EE (D)	K- Bechmanarth	Keelaparithiyur off-take near		
Er. V. Thiyagarajan, Dy.SE (D)	rajan, Dy.SE (D)		gar district	
Recommended by Er. K.S.K. Thulasiram, SE (D)	Sme and	General Not	és	
Approved by Er. 5, Anbazhagan, CE (DR&CS)	8000 16-11.1	Region: Madurai Region	Circle : Vaippar Basin Circle, Virudhunagar	
Drg. No.27	2/2012	Approved by CE (D AEE III / F563 / 2012	R&C5) in Lr No. 246 CE / SE(D) / t dated ) 6.11.2012	

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#### DESIGN OF PERMEABLE FOUNDATION

Permeable foundation is proposed for the anicut. The apron floor is designed for the following flow conditions.

1. Subsurface flow condition

2. Surface flow condition

The thickness of the floor is arrived based on Khosla's theory for the subsurface flow condition adopting an exit gradient of 1 in 6. The energy dissipation arrangements such as basin level, basin length are arrived for the surface flow conditions. The apron floor and cut off are proposed in M15 concrete. Wearing coat to a thickness of 25cm is proposed in the stilling basin portion in M20 concrete.

#### The salient features of the anicut are

Maximum Flood Discharge	:	301.572 Cumecs or 10650 Cusecs
Crest Level		+ 61.400m
Head over crest	:	2.18m
Front Maximum Water Level	:	+63.580m
Rear Maximum Water Level		+63.060m
Upstream bed level	:	+60.500m
Downstream bed level	:	+60.500m
Length of anicut	1.	70.0m
Height of anicut	:	0.90m
Top width	:	1.50 m
Upstream slope	:	0.1:1
Downstream slope	:	0.30:1
Total length of floor	:	12.50m
Depth of upstream cutoff	:	1.65m
Depth of downstream cutoff	:	1.91m
Thickness of floor at toe	:	1.00m
Thickness of floor at end	:	0.60m

Length of stilling basin	:	7.95m
Stilling Basin Level	:	+60.200m

Beyond the stilling basin 4 rows of CC blocks of size 1.5x1.5x0.9m in M15 concrete over an inverted filter of 60cm thickness are proposed. It is proposed to have a launching apron for a length of 8m having inner and outer thickness of 1.45m and 2.20m respectively.

## DESIGN OF SCOUR VENT PORTION

## Computation of linear waterway

It is proposed to provide 6 vents of size  $1.80 \times 0.90m$  (3 nos. on each sides) to discharge 19.31Cumecs or 681.93cusecs (i.e., 6.4% of the Maximum Flood Discharge) with sill level at +60.500 m.

# Design of permeable foundation

The apron floor is designed similar to the anicut portion. The apron floor and cut off are proposed in M15 concrete. Wearing coat to a thickness of 25cm is proposed in the stilling basin portion in M20 concrete

The salient features of the scour vent are as follows.

Discharge through scour vent	:	19.31 Cumecs or 681.93 Cusecs
Sill level	:	+47.20m
No. of vents	:	6 Nos.
Size of vent	:	1.80 x 0.90m
Total length of floor	:	12.50 m
Upstream Bed Level	, :	+60.500m
Downstream Bed level	:	+60.500m
Depth of upstream cutoff		1.65m
Depth of downstream cutoff		: 1.91m
Thickness of floor at toe		: 0.70m
Thickness of floor at end		: 0.60m
Stilling basin level		: +60.200m

Length of stilling basin	1	5.85
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Beyond the stilling basin 4 rows of CC blocks of size 1.5x1.5x0.9m in M15 concrete over an inverted filter of 60cm thickness are proposed. It is proposed to have a launching apron for a length of 8m having inner and outer thickness of 1.45m and 2.20m respectively.

#### DESIGN OF PIER

The top of the pier is kept as +64.180m. The height of the pier from the sill level is 3.68m. The stability of the pier is checked for the following conditions.

Condition1: All shutters are in closed condition

Condition2: All shutters are in fully opened condition

Condition3: When one shutter closed and the adjacent being in opened condition.

It is suggested to provide a thickness of 1.0m and a length of 2.50 m for the pier. The stresses developed due to the various forces involved at the four corners of the pier are computed for all the loading conditions and are summarized below.

SI.No	Condition	Stresses in t/m <sup>2</sup>					
		Corner A	Corner B	Corner C	Corner D		
1	All shutters are in closed condition.	14.042	3.096	3.096	14.042		
2	All shutters are in fully opened condition.	1.463	3.39	10.936	8.745		
3	When one shutter closed and the adjacent being in opened condition	6.72	4.969	5.192	7.207		

The pier is proposed in M15 concrete. Surface reinforcement with 8mm dia bars shall be provided in each direction.<sup>4</sup> le both horizontally and vertically. Spacing of such bars shall not exceed 200m.

#### DESIGN OF ABUTMENT AND WING WALLS

The left and right abutments are designed for the earth pressure caused due to the backfill material. The top of operating platform is proposed at +64,580m. The parameters of the backfill material such as saturated unit weight and angle of internal

friction have been assumed as  $2.0t/m^3$  and  $30^9$  respectively. Necessary wing walls have also been designed at the upstream and downstream sides.

The maximum stress developed at the sill level of the abutment is  $17.30 \text{ t/m}^2$ . The abutments and wing walls are proposed in M15 concrete using graded metal. It is suggested to provide surface reinforcement at the rate of 2.5 kg/m2 in the abutment wing walls and return walls at the water face in each direction. ie. both horizontally and vertically. Spacing of such bars shall not exceed 200 mm.

# DESIGN OF CANAL HEAD SLUICE

# Computation of linear waterway

It is proposed to provide 2 vents of size  $1.30 \times 0.60m$  to draw the required discharge of 2.294 Cumecs or 81 Cusecs, with sill level at +60.800m.

# Design of permeable foundation:

The apron floor is designed similar to the anicut portion. The apron floor and cut off are proposed in M15 concrete. Wearing coat to a thickness of 25cm is proposed in the stilling basin portion in M20 concrete

Discharge through Head Sluice vent	T	1 2 204 0
Sill level	-	· 2.294 Cumecs or 81 Cusecs
		: +60.800m
FSL in channel		: +61.07
No. of vents		: 2Nos.
Size of vent		1.30x0.60 m
Total length of floor	1	15.50m
Upstream Bed Level		+60.500m
Downstream Bed Level	-	+60.570m
Depth of upstream cutoff	1	1.65m
Depth of downstream cutoff	1.	2.50
Thickness of floor at toe		5.50m
Thickness of floor at end	-	1.60m
Skiller	:	1.30m
buing basin level	:	+59.87
ength of stilling basin	:	7.31m

# The salient features of the Head Sluice are as follows

#### Design of breast wall

Since the computed FMFL of +63.580m is higher than the top of shutter +61.550, breast wall is proposed in the sluice vent. Necessary top and bottom beams are proposed in the breast wall. The thickness of the proposed breast wall is 0.20m. The sizes of top and bottom beams are  $0.2m \times 0.20m$  and  $0.25m \times 0.25m$  respectively. The breast wall and beams are proposed in M25 concrete.

#### **Downstream Protection works**

Beyond the stilling basin 1 row of CC block of size 1.5x1.5x0.9m in M15 concrete over an inverted filter of 60cm thickness is proposed. It is proposed to have a launching apron for a length of 3.0m having inner and outer thickness of 1.20m and 1.90m respectively.

#### Design of Abutment and Wing Walls

The left and right abutments are designed for the earth pressure caused due to the backfill material. The top of operating platform is proposed at +64.580m and thus the top of the abutment is proposed at +64.580m. The parameters of the backfill material such as saturated unit weight and angle of internal friction have been assumed as  $2.0t/m^3$  and  $30^0$  respectively. Necessary wing walls are also designed at the upstream and downstream sides.

The maximum stress developed at the sill level of the abutment is 17.22 t/m<sup>2</sup>. The abutments and wing walls are proposed in M15 concrete using graded metal. It is suggested to provide surface reinforcement with 8mm dia bars in the abutment wing walls and return walls at the water face in each direction, ie. both horizontally and vertically. Spacing of such bars shall not exceed 200 mm.

#### The following points should be considered before taking up the work.

- The bottom of the D/s cut off wall shall not be keyed into the impervious layer as it will block the release of uplift pressure. If impervious layer is met with at the bottom of D/s cutoff, suitable filter arrangements have to be provided around the cutoff wall.
- Weep holes with necessary filter arrangements shall be provided at the upstream and downstream wing walls above the M.W.L. and R.W.L. respectively.
- The depth of footing for abutment and wing walls proposed below the sill level is as per the details furnished.

- 4) Surface reinforcement with 8mm dia bars shall be provided in the abutment and wing walls at the water face in each direction. ie. Both horizontally and vertically. Spacing of such bars shall not exceed 200 mm.
- Suitable Flood banks should be formed on the upstream and downstream side allowing a minimum free board of 1.0m above the water surface elevation.
- The pier should be constructed monolithic with the apron floor.

Copies of the approved drawings of components of the anicut are sent herewith for necessary action.

Enclosure: Drawings - 9 Nos

[ Sd----- ] Chief Engineer, PWD, DR&CS, WRO, Chennai -600 005.

Copy to the Superintending Engineer, PWD, Designs Circle, WRO, Chennai 5. Enclosure: Plan-9 Nos

Copy to the Superintending Engineer, PWD, Vaippar Basin Circle, Virudhunagar Enclosure: Plan-9 Nos

Copy to the Executive Engineer, PWD, Special Project Division, WRO, Madurai. Enclosure: Plan-9 Nos

Spare copy to CE, DR&CS office file with enclosure. Enclosure: Plan-9 Nos

11/13

for Chief Engineer, PWD, DR&CS, WRO, Chennai -600 005.















#### **General Notes:**

#### I. Anicut

- 1. The analysis has been made based on IS 6512-1984, IS 6966-1989, IS 12720-1993, IS 6531-1994, IS 1893-2002, IS 3370 - 2009 and IRC 6-2000.
- 2. The design has been formulated based on the particulars furnished by the Executive Engineer, Special Project Division, Madurai through E mail dated 27.11.12 & 14.12.2012 respectively.
- 3. The anicut has been designed for a maximum flood discharge of 10,650 cusecs.
- The length of the anicut between abuments is taken as 70 m whereas the width of river as per design communicated vide T.O. lr.mo.299CE/SE(D)/AEE X/F-563/dt.27.12.12. Hence the actual width of river during execution may be verified and the design of the anicut may be modified by referring to this office.
- 5. The parameters of backfill material such as saturated unit weight and angle of internal friction have been assumed as 2 t/cu.m. and 30° respectively. Hence suitable backfill material may be used accordingly.
- 6. The depth of footing for abutment and wingwalls proposed below the sill levels are tentative and may be suitably modified according to the site conditions.
- 7. The proposed lengths of upstream and downstream returns are tentative and may be modified to suit the site conditions
- 8. The apron floor, body wall, cut-off walls, pier, abutment, wingwalls and returns are proposed in M15 grade of concrete.
- 9. Surface reinforcement at the rate of 2.5 kg/sq.m. shall be provided in the pier, abutment, wing walls and returns at water face in each direction i.e., both horizontally and vertically. Spacing of such bars shall not exceed 200mm
- 10. The bottom of the D/s cutoff should not be keyed into the impervious layer as it will block the release of uplift pressure. If impervious layer is met with at the bottom of D/s cutoff, suitable filter arrangements should be provided around the cutoff.
- 11. Weep holes with necessary filter arrangements should be provided in the upstream and downstream wingwalls above the FMWL and RWL respectively.
- 12. Suitable Flood banks should be formed allowing a minimum free board of 1.0m above the water surface elevation in the both upstream and downstream side.
- 13. The maximum stress developed at the sill level of the abutment of anicut 16.63 t/sq.m. The SBC of the foundation media should be checked before taking up the work.
- 14. The proposed downstream protection works should be maintained periodically for effective functioning of the anicut.
- 15. The design for the operating platform and the structural details of the shutters are to be finalised by the Public Works Workshop
- 16. The top of operating platform has been tentatively fixed as +64.580 m assuming that the depth of beam and the thickness of chequered plates over the bed block is 0.40 m. However it may vary as per the design obtained from P.W.W.S. If there is any major variation, it shall be referred to Designs Circle for revision.
- 17. The Pier should be constructed monolithic with apron floor.
- 18. Transverse contraction joints with PVC water stops shall be provided in the anicut portion at every 25m interval at the locations specified in the drawing.
- 19. The bed level of the river at the D/s of the anicut and scour vent portions should be regraded from the proposed bed level of +60.500 m to a slope of 1 in 2200 for a width of 70 m atleast for a distance of 500 m. 20. The scour vents should be in fully opened condition during floods.
- 21. A clear cover of 75 mm shall be provided for the reinforcement provided in the stilling basin portion.
- 22. The structure is designed as Permeable foundation. During execution, if impervious strata found, the Design Circle shall be referred for revised design.
- 23. In general design the Girudhmal river, the downstram bed level of the Varisaiyur anicut is taken as 60.50 instead of 60.30 communicted early vide T.O.Ir.no.299CE/SE(D)/AEE X/F-563/2012/dt.27.12.12.. Hence the FSD changed from 2.60m to 2.56m
- 24. Assuming that the swell pressure is 50KN/Sqm. CNS Layer for the depth of 75cm is provided below the apron floor. Check the swell pressure before execution and depth of CNS layer to be provided as per Clause 4.2.5, of IS 9451:1994.

#### II. Head Shuice

- 1. The Head sluice has been designed (or a discharge of 81 cuseds (2.29 comeda)
- 2. The Canal bed width is 4.50m with a slpe of 1:1 and the bed fail is 1 in 600
- 3. Four number of vents of size 1.30 m x 0.60 m is proposed for the head stuice portion.
- 4. The energy dissipation arrangements for the head sluice portion have been designed for a discharge of 81 cusees. Therefore the vent opening should be so adjusted to maintain the discharge of 81 cusees in the canal by keeping the Full Supply Depth (FSD) of 0.50 m at Maximum Flood Condition.
- 5. Necessary transition may be provided at the end of the head sluice portion to negotiate the bed width of canal and the side slope
- 6. The bottom of the D/s cutoff should not be keyed into the impervious layer as it will block the release of uplift pressure. If impervious layer is met with at the bottom of D/s cutoff, suitable filter arrangements should be provided around the cutoff.
- 7. Assuming that the swell pressure is 50KN/Sqm. CNS Layer for the depth of 75 cm is provided below the apron floor. Check the swell pressure before execution and depth of CNS layer to be provided as per Clause 4.2.5. of IS 9451.1994.

This drawing should be road alongwith The drawings 373/2013 to 401/2013

Sheet No. 8/9

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Verified by Er. K. Padmanabhan, EE (D) Er. V. Thiyagarajan, Dy. SE (D)	K-Padmin AND	Varisaiyur ofi-takonear varisaiyur or Thiruchuli taluk of Virudhunagar district			
Recommended by Er. K.S.K. Thulasiram, SE (D)	Germans Into		General Notes		
Approved by Er. S. Anhazhágan, CE (DR&CS)	8 min 4-1-12	Region Madural Region	Circle : Vaippar Basin Circle. Virodhunagar		
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Public Works Department Water Resources Organisation

From Er.S. Anbazhagan, B.E., Chief Engineer, PWD Design Research and Construction Support, WRO, Chennai 600 005. To The Chief Engineer, PWD, Madurai Region, WRO, Madurai.

### Lr.No: CE / AEE X / F -563 / 2012 dated # 4 -01.2013

#### Sir,

Sub: Restoration and Regradation of Gridhumal river in Sivagangai, Virudhunagar an Ramanathapuram districts - Construction of Anicut across Giruthumal river to fee Kallikulam anicut near Kallikulam village of Kamuthi Taluk in Ramnad District Approved design and drawings of Anicut communicated – reg.

Ref: 1.G.O.Ms.No.269 PW(WR1)/dt.23.11.12.

2.Minutes of the Preliminary meeting of the Committee for "Assessment of le out works in IAMWARM sub basins on 26.11.12

3.EE, Spl. Pro. Divn. Madurai mail dt.26.11.12 & 14.12.12

The Engineer-in-Chief has communicated a copy of minutes in the reference 2 cited wherein the design and drawings of 3 anicuts namely Varisiyur, T.Punavasal an Kallikulam prepared by the Chief Engineer, Madurai Region should be vetted by the Cl DR&CS.

Accordingly, spot levels of the proposed alignment site and the supply channel ar trial pit details have been received vide reference 2<sup>nd</sup> cited.

Based on the particulars received, various components of the anicut across
Gridhumal river to feed Kallikulam tank @ LS 48850 m was arrived as follows

Bed slope	13	1 in 1425
Bed width	:	60m -
Maximum discharge		12530 cusecs

#### MAXIMUM FLOOD DISCHARGE

The maximum flood discharge of the anicut has been arrived as 12530 Cusecs ar the same is taken as design flood of the anicut.

### COMPUTATION OF REAR WATER LEVEL

From the L.S. of the river furnished, the bed slope of the river is computed as 1 1425. The R.W.L. is computed for the regraded section of the river adopting this be slope and works out to be +49.000m.

#### LAYOUT

It is proposed to provide 6 Nos. of scour vents (3 Nos. on either sides of aniculor of size  $1.50 \times 0.90$  m with sill level of +47.20 m with a discharging capacity of 25.8 cumecs under maximum flood condition. Therefore, the anicut is proposed for a length 45.00 m to dispose of the balance discharge. The total length of the structure considered for design in between the abutments is 60m.

### **CREST LEVEL OF ANICUT**

The crest level has been fixed at +48.10 m. The average bed level of the river the proposed site location is +47.20m. Hence, the height of the anicut is 0.90m.

### COMPUTATION OF FRONT MAXIMUM WATER LEVEL

The front MWL for the above arrangements to dispose the design flood, adopting narrow crested weir formula ( $ha_{1,1}$ ,  $a_{2,1}$ ,  $C_{0,1}$ , olue of 0.625 for submerged condition), for the anicut is computed as +50.35m, with a head over crest of 2.25m.

### DESIGN OF ANICUT PORTION

### STABILITY OF BODY WALL

A narrow crested weir having a top width of 1.20m, with upstream slope of 0.1 and downstream slope of 0.75:1 is proposed for the body wall portion. The stability the body wall is checked for the following loading conditions,

1. Empty condition.

2. Water at MWL, and maximum tail water with uplift.

3. Water at crest level with full uplift, no tail water.

The stresses developed at the bottom of the body wall are computed and tabulate below.

SI.No	Description	Stress in t/	m <sup>2</sup>
		Maximum	Minimum
1.	Empty condition	2.45	1.03
2.	Water at MWL and maximum tail water	0.11	0.04
3.	Water at crest level with full uplift and no ta	il 1.45	1.17

### DESIGN OF PERMEABLE FOUNDATION

Permeable foundation is proposed for the anicut. The apron floor is designed for the following flow conditions.

- 1. Subsurface flow condition
- 2. Surface flow condition

The thickness of the floor is arrived based on Khosla's theory for the subsurface flow condition adopting an exit gradient of 1 in 6. The energy dissipation arrangement such as basin level, basin length are arrived for the surface flow conditions. The aprofloor and cut off are proposed in M15 concrete. Wearing coat to a thickness of 25cm proposed in the stilling basin portion in M20 concrete.

Maximum Flood Discharge	:	354.87 Cumecs or 12530 Cusecs
Crest Level	1. A. A.	
Head over crest	:	2.25m
Front Maximum Water Level		+50.350m
Rear Maximum Water Level	:	+49.000m
Upstream bed level	;	+47.200m
Downstream bed level	• •	+46.200m
Length of anicut	:	60.0m
Height of anicut	:	0.90m
Top width		1.20 m
Upstream slope	:	0.1 : 1
Downstream slope	1	0.75:1
Total length of floor	:	19.00m
Depth of upstream cutoff	:	2.00m
Depth of downstream cutoff	1	2.75m
Thickness of floor at toe	:	1.60m
Thickness of floor at end		0.90m

### The salient features of the anicut are

Length of stilling basin	: 13.31m
Stilling Basin Level	
Downstroam n	, T43./00m

Beyond the stilling basin 5 rows of CC blocks of size 1.5x1.5x0.9m in M15 concrete over an inverted filter of 60cm thickness are proposed. It is proposed to have a launching apron for a length of 10m having inner and outer thickness of 1.60m and 2.40m respectively.

# DESIGN OF SCOUR VENT PORTION

# Computation of linear waterway

It is proposed to provide 6 vents of size 1.50 x 0.90m (3 nos. on each sides) to discharge 25.87 Curnecs or 913.65 cusecs (i.e., 7.3% of the Maximum Flood Discharge) with sill level at +47.20 m.

# Design of permeable foundation

The apron floor is designed similar to the anicut portion. The apron floor and cut off are proposed in M15 concrete, Wearing coat to a thickness of 25cm is proposed in the stilling basin portion in M20 concrete.

# The salient features of the scour vent are as follows,

Discharge through scour vent		25.87 Cuman
Sill level	+	usion cullecs or 913.65 Cusecs
No, of vents	-	+47.20m
Cize of west	-	6 Nos.
Size of vent	:	1.50 x 0.90m
Total length of floor	;	19.00 m
Upstream Bed Level	:	+47.200m
Downstream Bed level	-	+46,200m
Depth of upstream cutoff		2.00m
Depth of downstream cutoff	-	275m
Thickness of floor at toe	1	1 20m
Thickness of floor at end	+	4.2010
Stilling bacin land	1	0.80m
sensing paper level	:	+45.900m

Length of stilling ba	sin :	9.35	3	

Beyond the stilling basin 5 rows of CC blocks of size 1.5x1.5x0.9m in concrete over an inverted filter of 60cm thickness are proposed. It is proposed to a launching apron for a length of 10m having inner and outer thickness of 1.60m 2.40m respectively.

#### DESIGN OF PIER

The top of the pier is kept as +50.95 m. The height of the pier from the sill is 3.75m. The stability of the pier is checked for the following conditions.

Condition1: All shutters are in closed condition

Condition2: All shutters are in fully opened condition

Condition3: When one shutter closed and the adjacent being in opened condition.

It is suggested to provide a thickness of 1.0m and a length of 2.50 m for the The stresses developed due to the various forces involved at the four corners of the are computed for all the loading conditions and are summarized below.

SI.No	Condition	Stresses in t/m <sup>2</sup>					
		Corner A	Corner B	Corner C	Corner D		
1	All shutters are in closed condition.	16.514	1.081	1.081	16.514		
2	All shutters are in fully opened condition.	1.658	3.173.	11.887	9.360		
3	When one shutter closed and the adjacent being in opened condition	7.586	2.663	3.805	9.740		

The pier is proposed in M15 concrete. Surface reinforcement with 8mm dian bars shall be provided in each direction. ie both horizontally and vertically. Spacin such bars shall not exceed 200m.

#### DESIGN OF DIVIDE WALL

It is suggested to provide divide wall at downstream side only in between scour vents and anicut portion. The proposed top levels of upstream and downstr divide wall are fixed as +48.10 m. The divide walls are designed based on IS 127 1993. The proposed thickness of the downstream divide wall is 0.60m. The divide w proposed in M25 concrete. Necessary reinforcements in Fe415 are proposed for moments developed.

#### DESIGN OF ABUTMENT AND WING WALLS

The left and right abutments are designed for the earth pressure caused due the backfill material. The top of operating platform is proposed at +51.350m. parameters of the backfill material such as saturated unit weight and angle of interfriction have been assumed as 2.0t/m<sup>3</sup> and 30<sup>0</sup> respectively. Necessary wing walls h also been designed at the upstream and downstream sides.

The maximum stress developed at the sill level of the abutment is 19.19 t/ The abutments and wing walls are proposed in M15 concrete using graded metal. I suggested to provide surface reinforcement with 8mm diameter bars in the abutm wing walls and return walls at the water face in each direction. ie. both horizontally vertically. Spacing of such bars shall not exceed 200 mm.

### **DESIGN OF CANAL HEAD SLUICE**

#### **Computation of linear waterway**

It is proposed to provide 1 vent of size  $0.70 \times 0.60$ m to draw the requidischarge of 0.595 Cumecs or 21 Cusecs, with sill level at +47.500m.

#### Design of permeable foundation:

The apron floor is designed similar to the anicut portion. The apron floor and off are proposed in M15 concrete. Wearing coat to a thickness of 25cm is proposed in stilling basin portion in M20 concrete

Discharge through Head Sluice vent		0.595 Cumecs or 21 Cusecs	
Sill level	:	+47.500m	
FSL in channel	:	+47.740m	
No. of vents	:	1No.	
Size of vent	:	0.70x0.60 m	
Total length of floor	:	16.00m	
Upstream Bed Level	:	+47.200m	
Downstream Bed Level	:	+47.500m	

#### The salient features of the Head Sluice are as follows

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Length of stilling basin	:	7.66m	
Stilling basin level	:	+46.500m	
Thickness of floor at end	:	1.50m	
Thickness of floor at toe	• :	1.80m	
Depth of downstream cutoff	:	3.10m	
Depth of upstream cutoff	:	1.65m	

### Design of breast wall

Since the computed FMFL of +5.350m is higher than the top of shutter +48.25 breast wall is proposed in the sluice vent. Necessary top and bottom beams a proposed in the breast wall. The thickness of the proposed breast wall is 0.20m. The sizes of top and bottom beams are 0.2m x 0.20m and 0.25m x 0.25m respectively. The breast wall and beams are proposed in M25 concrete.

### **Downstream Protection works**

Beyond the stilling basin 2 rows of CC block of size 1.5x1.5x0.9m in M15 concre over an inverted filter of 60cm thickness is proposed. It is proposed to have a launchir apron for a length of 3.0m having inner and outer thickness of 1.90m and 2.90 respectively. the bottom of D/s cutoff, suitable filter arrangements have to be provided around the cutoff wall.

 Weep holes with necessary filter arrangements shall be provided at the upstream and downstream wing walls above the M.W.L. and R.W.L. respectively.

- The depth of footing for abutment and wing walls proposed below the sill level is as per the details furnished.
- 4) Surface reinforcement with 8mm diameter bars shall be provided in the abutment and wing walls at the water face in each direction. ie. Both horizontally and vertically. Spacing of such bars shall not exceed 200 mm.
- Suitable Flood banks should be formed on the upstream and downstream side allowing a minimum free board of 1.0m above the water surface elevation.
- 6) The pier should be constructed monolithic with the apron floor.

Copies of the approved drawings of components of the anicut are sent herewith for necessary action.

Enclosure: Drawings - 9 Nos

-sd-Chief Engineer,PWD, DR&CS, WRO, Chennai -600 005

- Copy to the Superintending Engineer, PWD, Designs Circle, WRO, Chennai 5. Enclosure: Plan- 9 Nos
- Copy to the Superintending Engineer, PWD, Vaippar Basin Circle, Virudhunagar Enclosure: Plan- 9 Nos

Copy to the Executive Engineer, PWD, Special Project Division, WRO, Madurai. Enclosure: Plan- 9 Nos

Spare copy to CE, DR&CS office file with enclosure.

Enclosure: Plan- 9 Nos

for Chief Engineer,PWD, DR&CS, WRO, Chennai -600 005.









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T No 389/2012

## NOTES:

1. The analysis has been made based on IS 6512-1984. IS 6966-1989. IS

12720-1993, IS 6531-1994, IS 13551-1992, IS 11130 - 1984 and IS 1893 - 2002.

2. The design has been formulated based on the particulars furnished by the EE, Special project division, Madurai, through e-mail Dated 27.11.12 and 14.12.12 respectively.

3. The anicut has been designed for a maximum flood discharge of 12530 cusecs.

4. The parameters of backfill material such as saturated unit weight and angle of internal friction have been assumed as 2t/cum and 30° respectively.

5. The depth of footing for abutment and wingwalls proposed below the sill levels are tentative and may be suitably modified according to the site conditions.

6. The proposed lengths of upstream and downstream returns are tentative and may be modified to suit the site conditions.

7. The apron floor, body wall, pier, abutment, wing walls and returns are proposed in M15 concrete using graded metal.

8. Surface reinforcement at the rate of 2.5Kg/m2 shall be provided in the abutment, wing wall, returns and pier in each direction ie both horizontally and vertically. Spacing of such bars shall not exceed 200mm.

9. Weep holes with necessary filter arrangements should be provided in the upstream and downstream wingwalls above the FMWL and RWL respectively. 10. Suitable Flood banks should be formed on the upstream and

dowstream side allowing a minimum free board of 1.0m above the water surface elevation.

11. The maximum stress developed at the sill level of the downstream wing wall of anicut is 19.19/sg.m. The SBC of the foundation media should be checked before taking up the work.

12. The downstream protection works should be maintained periodically for effective functioning of the anicut.

13. The design for the operating platform and the structural details of the shutters are to be finalised by P.W.W.S.

14. The top of operating platform has been tentatively fixed as +51.350m assuming that the depth of beam and the thickness of chequered plates over the bed block is 0.40m. However it may vary as per the design This drawing has to be rea obtained from P.W.W.S. series of drawings T No 37

15. The scour vents shall be in fully opened condition during floods.

16. The pier should be constructed monolithic with the apron floor.

17. Transverse contraction joints with PVC water stops shall be provided in the anicut portion at every 25m interval and at the locations specified in the drawing.

18. D/s divide wall is to be constructed monolithic with the D/s apron floor upto the end of stilling basin of scour vent portion.

19. A clear cover of 75mm shall be provided for the reinforcement provided in the stilling basin portion of anicut, scour vent and canal sluice.

20. The bed level of river, at the D/S of the anicut portion should be regraded for the proposed bed level of +46.20m to a slope of 1 in 1425 for the river width of 60m atleast for a distance of 500m/

21. The canal sluice has been designed for a discharge of 21 cusecs. 22. The top and bottom beams of breast wall in canal sluice should be embedded into the pier for a depth of 0.3m on both sides.

23. Necessary transitions shall be made at the end of the canal sluice portion to negotiate the width of the canal sluice with the bed width of the canal.

24. The bed level of the canal at the offtake has been redesigned at +47,500m. The canal for supply channel is designed for bed width of 6.00m, side slope 1:1 with a bed slope of 1 in 1000. The canal ' may be regraded acordingly upto mouth into tank.

Sheet No 8/9

	GOVERNMENT OF TAMIL NADU PUBLIC WORKS DEPARTMENT OFFICE OF THE SUPERINTENDING ENGINEER, PWD, DESIGNS CIRCLE, WRO, CHENNAI - 600 005			
	Designed by Er.S.Poorni A.E.E	P3-31112/12	RESTORATION AND RECRADATION OF GRIDHMAL RIVER - CONSTRUCTION OF ANGUT ACROSS GRIDHMAL RIVER TO FRED KALLINULAM OFFTAKE LSA KALLINULAW VILLAGE OF KAMUTHI TALUK IN RAMNAD DISTRICT	4815
	Drawn by Er.H.Yogapriya A.E	4Mailian-		
	Checked by Er.S.Poorni A.E.E	Apaint		
	Verified by Er.K. Podmonabhan, EE (D) Er.V. Thiyagarajan, DSE	k-Padmanan	Designed Drawing of General notes	
ad with the 72/2012 to	Recommended by Er.K.S.K. Thulasiram, SE(D) Approved by S. Anhazhagan C.F. (DR&CS)	Sman 1	Region: Circle:Vaippor Basin MaduraiRegion Circle, Virudhunagar.	
	T.NO: 37 9/ 2012	SCALE 1:150	Approved by CE (DR&CS)in Lr No: %CE (DR&CS) /SE(D) / AEE X /F. 563/ 2012 dated :A L. L 5	1.5

